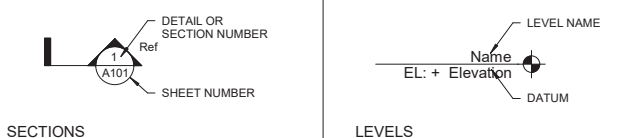
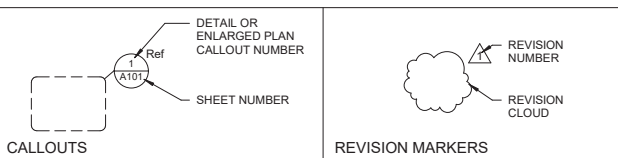


ATTACHMENT “B”

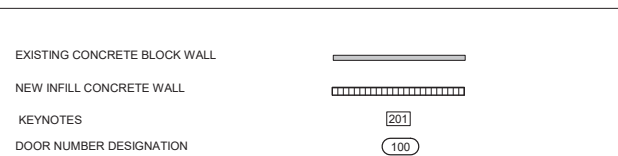
GENERAL NOTES FOR NEW CONSTRUCTION PLANS

- SEE FLOOR PLANS, ENLARGED PLANS, DEMOLITION AND INTERIOR ELEVATIONS, DETAILS, ETC., FOR DIMENSIONS AND KEYNOTES WHICH APPLY TO THE NEW CONSTRUCTION FLOOR PLANS.
- DIMENSIONS WITH +/- CAN BE LARGER OR SMALLER THAN SHOWN. CONTACT ARCHITECT IF DIMENSIONS WITH +/- VARY MORE THAN 1 INCH.
- DIMENSIONS WITHOUT +/- MUST BE CONSTRUCTED AS SHOWN.
- REPAIR EXISTING WALLS AS REQUIRED TO PROVIDE NEW APPEARANCE. APPLY FINISHES NOTED ON THE DRAWINGS.
- PATCH, REPAIR, AND LEVEL EXISTING FLOOR SLAB TO RECEIVE NEW FLOOR FINISH. REFER TO FINISH SCHEDULE. DIMENSIONS INDICATED AS "MIN (OR MAX) CLEAR" MUST BE MAINTAINED FROM FINISH SURFACE TO FINISH SURFACE.
- PATCH ALL HOLES IN EXISTING CONCRETE FLOOR SLAB AND CMU WALLS WHERE MECHANICAL, ELECTRICAL, OR OTHER PENETRATIONS HAVE BEEN REMOVED. REPAIRS SHALL MATCH EXISTING WALL AND THE FIRE RATING OF THE SURROUNDING ASSEMBLY.
- CONTRACTOR TO NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN FIELD DIMENSIONS AND PLANS.
- EXISTING-TO-REMAIN WALLS, VENTILATION PANELS, PLUMBING FIXTURES, EQUIPMENT, ETC., ARE SOLID GRAY ON THE NEW CONSTRUCTION FLOOR PLANS. NEW CONSTRUCTION IS INDICATED BY SOLID LINES. REFER TO DEMOLITION FLOOR PLANS FOR EXTENT OF "EXISTING-TO-BE-REMOVED" COMPONENTS AND ELEMENTS.
- ACCESS PANELS:
 - THE CONTRACTOR SHALL VERIFY THAT ACCESS HATCH TYPE JUSTIFIED IS INSTALLED ON ROOF AND IN "ACCESSIBLE TYPE" CEILING WHERE ACCESS, SERVICE OR ADJUSTMENT TO MECHANICAL, PLUMBING, FIRE PROTECTION, SECURITY, ELECTRICAL AND COMMUNICATION ITEMS MAY BE REQUIRED.
 - ACCESS HATCH SHALL BE THE "FIRE RATED TYPE" EQUAL TO THE RATING OF THE ROOF IN WHICH IT OCCURS.
 - ACCESS HATCH LOCATED IN FIRE RATED ROOF SHALL BE SELF-CLOSING AND "KEY LOCKED".
- THE DRAWINGS FOR THE ROOF HATCH IS A BASIS OF DESIGN ONLY TO COMPLY WITH PROJECT PROGRAMMATIC REQUIREMENTS. PRODUCT SHALL BE DESIGN BY A DELEGATED ENGINEER AND COMPLY WITH FBC FOR WIND LOADS AND IMPACT RATING.

REFERENCE SYMBOLS



LEGEND



PROJECT SCOPE OF WORK

- PARTIALLY REMOVE AND REPLACE COMPONENTS OF AN EXISTING WATER PUMP STATION.
- REMOVE AND REPLACE AN EXISTING ROOF FRAMING, LIGHT FIXTURES/SWITCHES, MECHANICAL EQUIPMENT/LOUVER GRILL, EXTERIOR DOOR/HARDWARE. ADD A NEW GRATE FLOOR SYSTEM, DOORFRAME WALL, LIGHTING AND ROOF.
- GENERAL CONTRACTOR SHALL COMPLY WITH ALL CURRENT APPLICABLE CODES.
- GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS/SAMPLE SUBMITTALS FOR ALL MATERIALS/PRODUCTS SPECIFIED FOR APPROVAL PRIOR TO PROCEEDING WITH THE SCOPE WORK.

APPLICABLE CODES

FLORIDA BUILDING CODE, BUILDING 2020
 FLORIDA BUILDING CODE, EXISTING BUILDING 2020
 FLORIDA ACCESSIBILITY CODE 2012 BASED ON 2010 ADA STANDARDS
 FLORIDA BUILDING CODE, MECHANICAL 2020
 FLORIDA BUILDING CODE, ELECTRICAL 2020
 FLORIDA FIRE PREVENTION CODE 2020
 NFPA 101- LIFE SAFETY CODE, 2018
 NFPA 1- FIRE CODE, 2018
 NFPA 72- FIRE ALARM AND SIGNALING CODE, 2016
 NFPA 13- STANDARD FOR THE INSTALLATION OF SPRINKLER SYSTEMS, 2016
 NFPA 70- NATIONAL ELECTRIC CODE, 2018
 NATIONAL ELECTRICAL CODE, 2017
 CURRENT OSHA REGULATIONS

PROJECT GENERAL NOTES

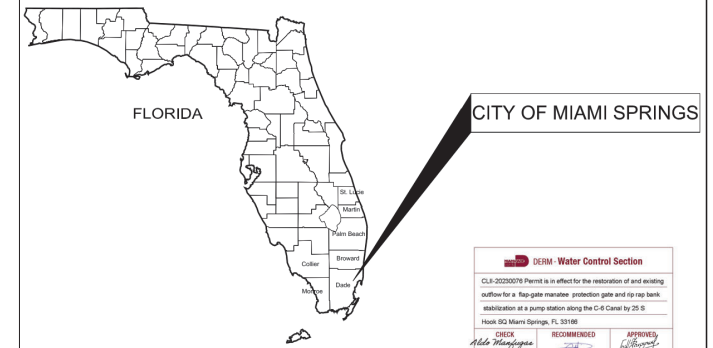
- ALL WORK TO BE PERFORMED UNDER THIS CONTRACT SHALL COMPLY WITH, BUT IS NOT LIMITED TO, CODES LISTED WITHIN THIS PROJECT INCLUDING THE FOLLOWING: THE FLORIDA BUILDING CODE (LATEST EDITION), CITY LABOR LAWS, CITY ORDINANCES (CITY/COUNTY ZONING CODES, NATIONAL ELECTRICAL CODE, N.F.P.A. 101, O.S.H.A., A.D.A. & ALL OTHER APPLICABLE CODES, RULES AND REGULATIONS.
- THE CONTRACTOR SHALL FAMILIARIZE HIM/HERSELF WITH THE PROJECT THROUGH INSPECTION OF THE SITE, THE DRAWINGS AND SPECIFICATIONS, SO AS TO THOROUGHLY UNDERSTAND THE NATURE AND SCOPE OF THE WORK. ANY AMBIGUITIES OR CONFLICTS WITH THE CONSTRUCTION DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT IN WRITING PRIOR TO PROVIDING COST OF CONSTRUCTION.
- IN THE EVENT THAT CERTAIN FEATURES OF THE CONSTRUCTION ARE NOT FULLY SHOWN ON THE DRAWINGS, THEN THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS FOR SIMILAR CONDITIONS THAT ARE SHOWN OR NOTED AND SHALL BE INCLUDED AS PART OF COST OF CONSTRUCTION.
- EXISTING FACILITY ITEMS AND EQUIPMENT THAT ARE NOT PART OF SCOPE OF WORK ARE TO REMAIN INTACT. HOWEVER, CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF SOME ADJACENT ITEMS AND EQUIPMENT WHICH ARE NOT PART OF THE SCOPE OF WORK IN ORDER TO ACCOMPLISH DEMOLITION AND NEW CONSTRUCTION WORK. FURTHER MORE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REINSTALLATION OF THOSE ADJACENT ITEMS AND EQUIPMENT AND THEY SHALL BE IN PROPER WORKING CONDITION.
- ALL DIMENSIONS PROVIDED ARE APPROXIMATE. CONTRACTOR SHALL VERIFY DIMENSIONS WITH EXISTING FIELD CONDITIONS. ANY DISCREPANCIES SHALL BE BROUGHT TO ARCHITECT'S ATTENTION BEFORE COMMENCEMENT AND/OR PROCEEDING WITH THE WORK.
- CONTRACTOR SHALL PROTECT ALL EXISTING FLOOR & WALL SURFACES WITH A MEANS OF CONTAINMENT TO PREVENT DAMAGE.
- ALL EXISTING SURFACES ARE TO BE PRIMER-COATED IN ADVANCE OF PAINT FINISH PER SPECIFICATION.
- THE CONTRACTOR SHALL MAINTAIN ALL COMPONENTS FOR MEANS OF EGRESS FREE OF OBSTRUCTIONS AND ACCESSIBLE AT ALL TIMES.
- THE CONTRACTOR SHALL MAINTAIN ALL LIFE SAFETY AND FIRE PROTECTION SYSTEMS OPERATIONAL AT ALL TIMES.
- CONTRACTOR SHALL MAINTAIN ALL EXISTING FIRE RATINGS AT NEW CONSTRUCTION.
- ANY FIRE RATED COMPONENT, SYSTEM, OR MATERIAL THAT IS REMOVED IN THE COURSE OF CONSTRUCTION MUST BE REPLACED OR UPDATED AS PART OF THE SCOPE OF WORK FOR THIS PROJECT.
- CONSTRUCTION ACTIVITY AFTER REGULAR WORKING HOURS MUST BE COORDINATED AND APPROVED BY THE OWNER.
- CONTRACTOR AND ALL SUBCONTRACTORS SHALL FIELD CHECK AND VERIFY ALL EXISTING CONDITIONS AND SHALL BE RESPONSIBLE FOR THEIR PRESERVATION. ANY DAMAGE TO EXISTING SHALL BE REPAIRED AND/OR REPLACED AT NO ADDITIONAL COST TO OWNER.
- CONTRACTOR MAY NEED TO PENETRATE OR OTHERWISE MODIFY FIRE RATED WALLS. COORDINATE SCOPE OF MECHANICAL AND ELECTRICAL TO DETERMINE LOCATIONS. WALL RATINGS SHOULD BE PRESERVED AT ALL TIMES. PENETRATIONS SHALL BE PROPERLY SEALED.
- CONTRACTOR TO PROVIDE LIFE SAFETY MEASURES FOR THE CONSTRUCTION AREA ITSELF, INCLUDING UNOBSTRUCTED MEANS OF EGRESS, FIRE EXTINGUISHERS, AND MINIMUM LIGHTING LEVELS. ADDITIONAL MEASURES MAY BE REQUIRED BY THE OWNERS.

GENERAL NOTES FOR DEMOLITION PLANS

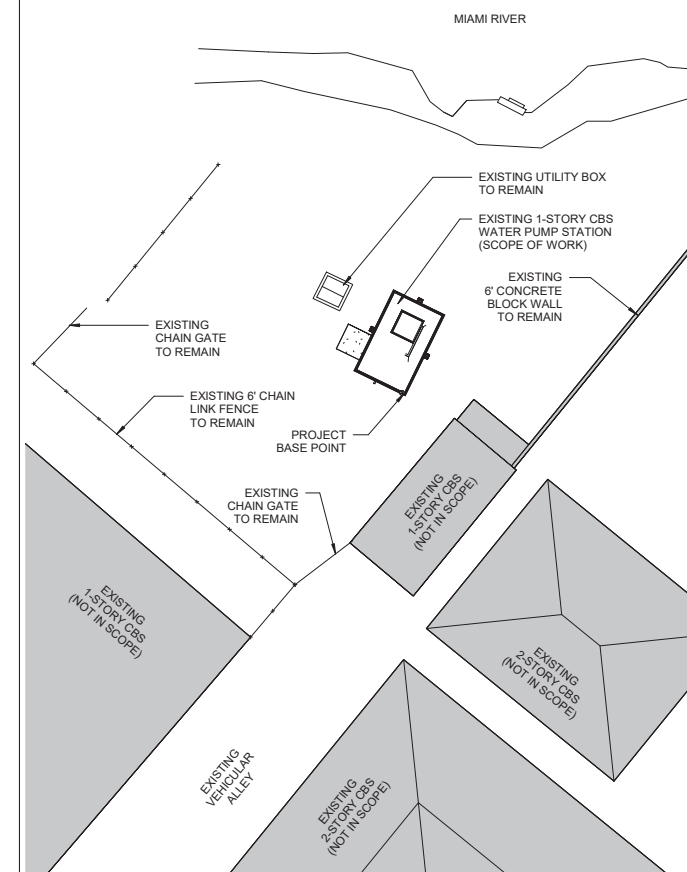
- NO WORK IN SHADED AREAS (TYPICAL UNLESS OTHERWISE NOTED)
- REMOVE ALL DASHED WALLS, DOORS, FRAMES, COMPONENTS, BUILDING EQUIPMENT AND FIXTURES IN THE WORK AREA PRIOR TO START OF NEW CONSTRUCTION.
- REMOVE DESIGNATED EXISTING BUILDING COMPONENTS AS INDICATED. PATCH AND REPAIR ALL DAMAGED AREAS AND PREPARE BASE BUILDING SURFACES FOR INSTALLATION OF NEW MATERIALS.
- EXISTING CMU WALLS AND VENTILATION PANELS TO BE PROTECTED PROPERLY FROM DAMAGE.
- DEMOLITION OF EXISTING MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION SYSTEM, EXISTING ROOF/WOOD FRAME SYSTEM TO BE COORDINATED WITH M.E.P. DEMOLITION DRAWINGS.
- FIELD VERIFY WITH ENGINEER EXTENT OF DEMOLITION SCOPE FOR EXISTING MECHANICAL, ELECTRICAL, PLUMBING AND FIRE PROTECTION SYSTEM ELEMENTS.
- CONTRACTOR TO ENSURE CLEAN UNINTERRUPTED ACCESS TO FIRE EXIT GATES DURING CONSTRUCTION.

DRAWING INDEX

SHEET NUMBER	SHEET DESCRIPTION	PERMIT SET	REV 01	REV 02	REV 03
		DATE ISSUED: 10/11/23	DATE ISSUED:	DATE ISSUED:	DATE ISSUED:
GENERAL					
COVER	COVER SHEET	•			
G001	SITE PLAN, GENERAL NOTES & INDEX OF DRAWINGS	•			
CIVIL / STRUCTURE					
C-1	SITE & REVETMENT PLAN	•			
C-2	DRAINAGE DETAILS	•			
FLOOR PLANS & RCPS					
A201	FLOOR PLANS, RCPS, ELEVATIONS & SECTIONS	•			
DETAILS					
A800	DOOR, WALL, ROOF DETAILS AND SCHEDULES	•			
STRUCTURAL					
S-1	STRUCTURAL & MECHANICAL DEMOLITION	•			
S-2	DEMOLITION PHOTOS	•			
S-3	ROOF DETAILS	•			
S-4	FRAMING DETAILS	•			
MECHANICAL					
M-1	PUMP INSATALLATION DETAILS	•			
M-2	HOOD AND EXHAUST FAN DETAILS	•			
ELECTRICAL					
E-1	ELECTRICAL GENERAL NOTES AND LEGEND	•			
E-2	ELECTRICAL FLOOR PLAN	•			



PROJECT LOCATION
N.T.S.



SITE PLAN
1/16" = 1'-0"



CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
 4711 South LeJeune Road
 Coral Gables, FL 33146
 P: 305.859.2050

STRUCTURAL / MEP ENGINEER:
R.J. Behar & Company, Inc.
 Engineers - Planners
 6861 SW 196th Ave suite 302,
 Fort Lauderdale, FL 33332
 (954) 680-7771

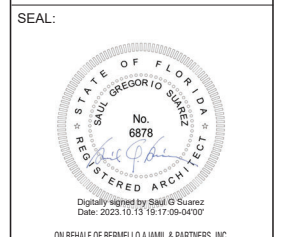
OWNER:
CITY OF MIAMI SPRINGS
 201 WESTWARD DR
 MIAMI SPRINGS, FL 33166



**MIAMI SPRINGS
 HOOK SQUARE
 PUMP STATION**
 25 S. HOOK SQUARE
 MIAMI SPRINGS, FL 33166

PHASE:
PERMIT SET

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REVISIONS:

No.:	DESCRIPTION	DATE:

TITLE:
**SITE PLAN, GENERAL
 NOTES & INDEX OF
 DRAWINGS**

Project No: 02296.000
 Date: 8/4/2023
 Scale: As indicated
 Format: 24" x 36"
 Drawn: JFG
 Checked: JF

SHEET:
G001



NORHT VIEW



SOUTH VIEW



WATER PUMP STATION



EXISTING ROOF

MIAMI SPRINGS HOOK SQUARE PUMP STATION

25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

PERMIT SET

OCTOBER 11, 2023

Project Number: 02296.000

CITY OF MIAMI SPRINGS



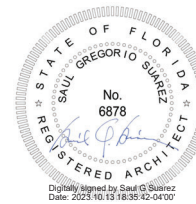
201 WESTWARD DR
MIAMI SPRINGS, FL 33166

ARCHITECT



BERMELLO AJAMIL & PARTNERS
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

ARCHITECT'S SEAL



CIVIL ENGINEER



BERMELLO AJAMIL & PARTNERS
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

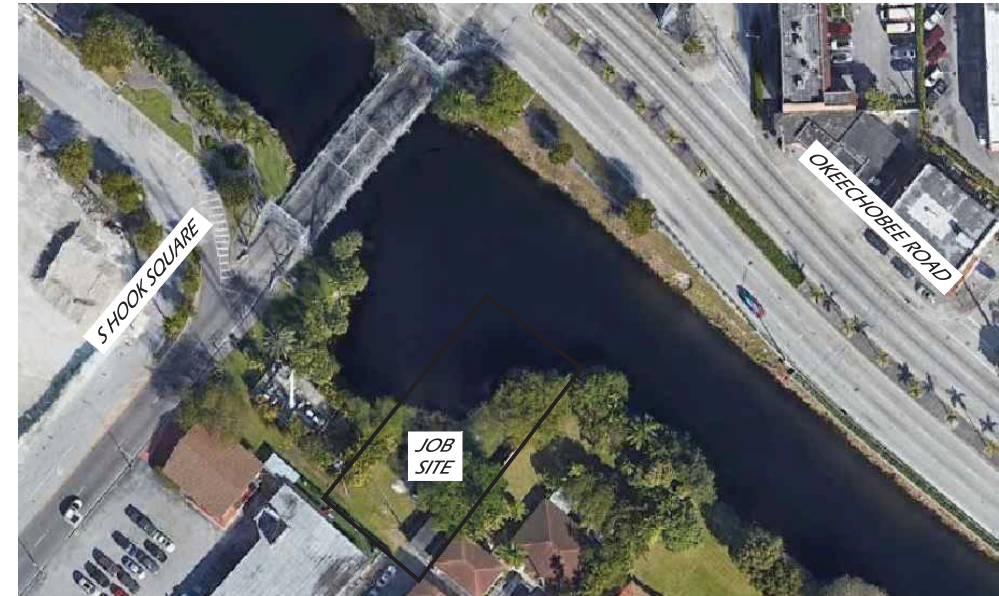
STRUCTURAL / MEP
ENGINEER



R.J. Behar & Company, Inc.
Engineers • Planners

6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

SPECIFIC PURPOSE SURVEY



LOCATION SKETCH
NOT TO SCALE

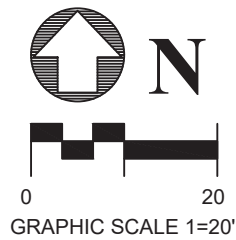
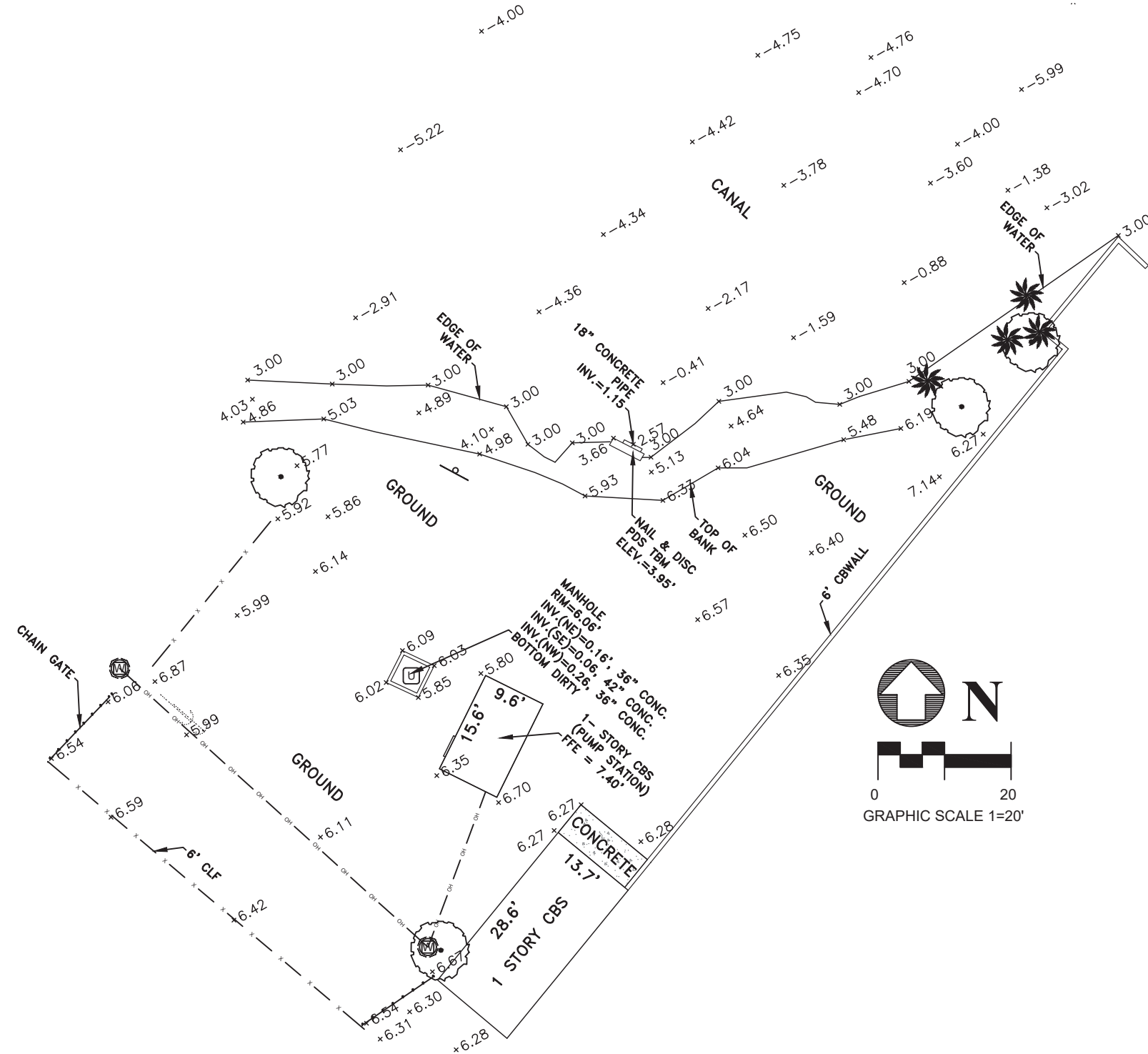
PROPERTY ADDRESS
25 S HOOK SQ, CITY OF MIAMI SPRINGS, FLORIDA 33166

PREPARED FOR:
CITY OF MIAMI SPRINGS, FLORIDA

- SURVEYOR'S NOTES:
1. NO UNDERGROUND PORTIONS OF FOOTINGS, FOUNDATIONS. CONTACT APPROPRIATE AUTHORITY PRIOR TO ANY DESIGN WORK OR CONSTRUCTION.
 2. THIS IS NOT A BOUNDARY SURVEY. EXAMINATION OF ABSTRACT OF TITLE MUST BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING PROPERTY BESIDES THOSE NOTED.
 3. NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.
 4. TYPE OF SURVEY: SPECIFIC PURPOSE.
 7. ELEVATIONS ARE REFERRED TO THE NATIONAL GEODETIC VERTICAL DATUM 1929. USED BENCHMARK: M-75-1. ELEVATION: 7.61 (NORTH GEODETIC VERTICAL DATUM 1929)
 8. FIELD WORK DATE: 01.20.2023

SYMBOLS & ABBREVIATIONS
CBS = CONCRETE BLOCK STRUCTURE; CBWALL= CONCRETE BLOCK WALL; CL = CENTER LINE; CLF = CHAIN LINK FENCE; EOP = EDGE OF PAVEMENT; FND= FOUND; FFE = FINISH FLOOR ELEVATION; I.D.= IDENTIFICATION; INV.= INVERT ELEVATION; IP = IRON PIPE; MDCR = MIAMI DADE COUNTY RECORDS; NAC = NON ACCESSIBLE CORNER; PB = PLAT BOOK; PG = PAGE; RB = REBAR; RDS= PREMIER DESIGN SOLUTIONS; UE = UTILITY EASEMENT;

NOTE: SYMBOLS SHOWN ARE NOT TO SCALE AND SHALL NOT BE USED TO SIZE SUCH ELEMENTS.



PREMIER DESIGN SOLUTIONS INC.
11606 CITY HALL PROMENADE • STE 200
• MIRAMAR FL, 33025
954.237.7850
PDS@PDS-ENG.COM
FLORIDA COA No. 27940
LB 8017

REVISIONS			
No.	DATE	REMARKS	BY

CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166

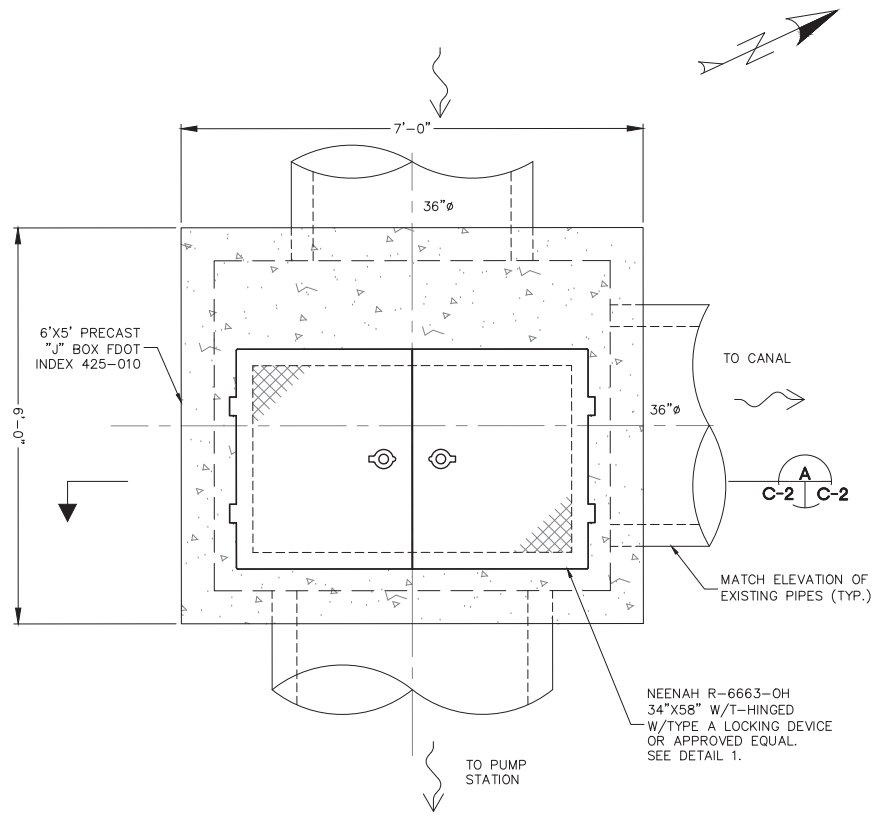


SPECIFIC PURPOSE SURVEY

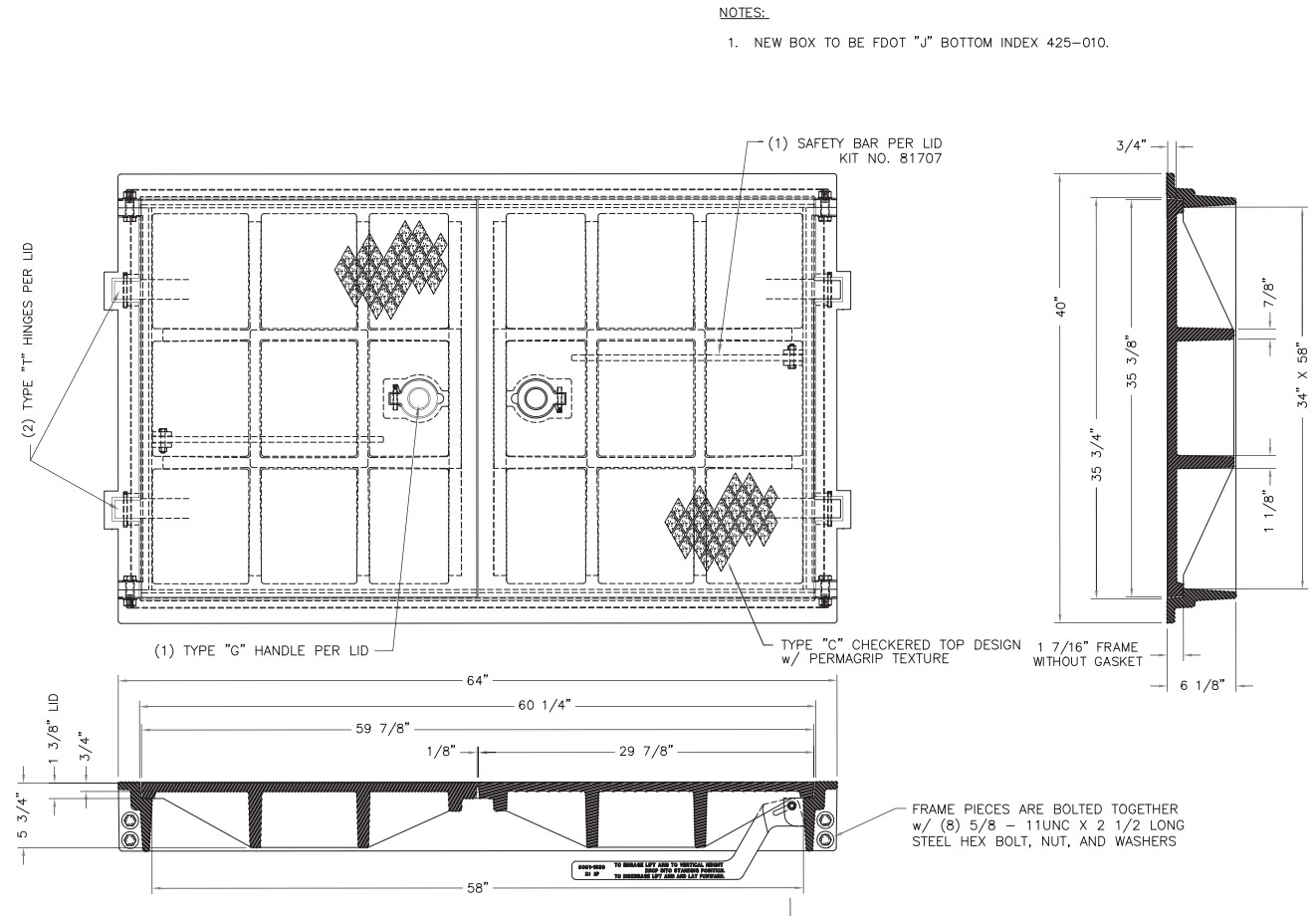
25 S HOOK SQ
MIAMI SPRINGS, FL 33166

FERNANDO FERNANDEZ
PROFESSIONAL SURVEYOR AND MAPPER
LICENSE LS-6765 STATE OF FLORIDA
ffernandez@pds-eng.com

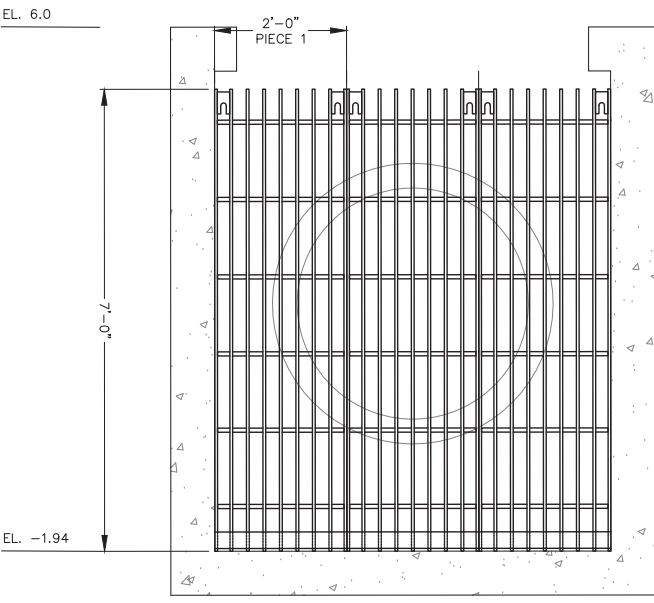
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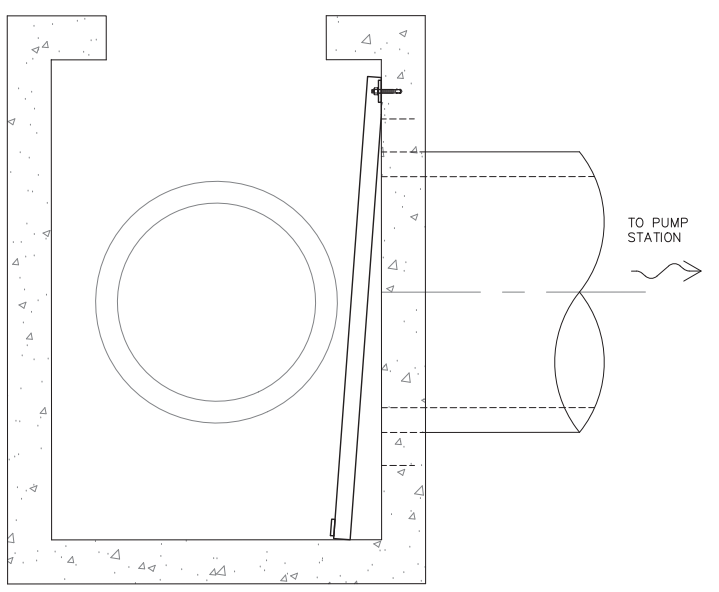
INFLOW BOX ACCESS PLAN VIEW
SCALE: 3/4" = 1'-0"



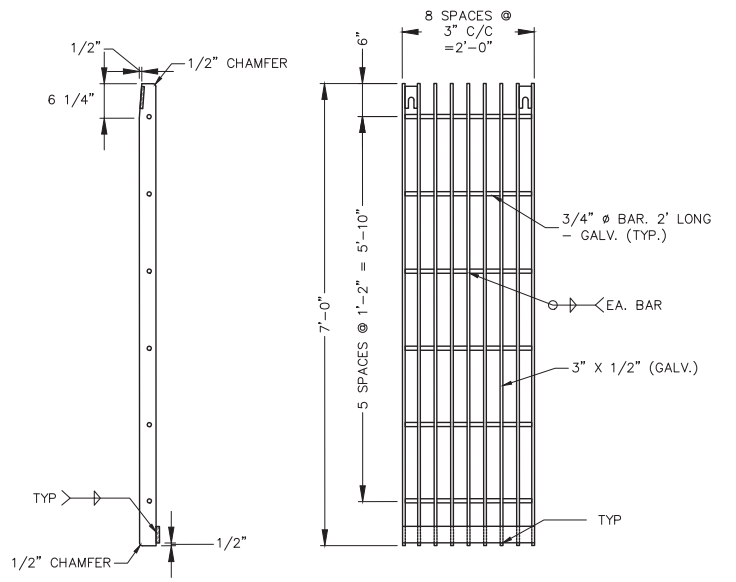
NEENAH R-6663-OH FRAMES AND LIDS DETAIL
SCALE: N.T.S.



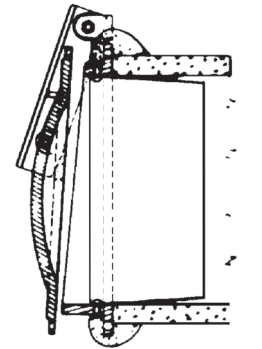
TRASH RAKE ELEVATION
SCALE: 3/4" = 1'-0"



TRASH RAKE SIDE VIEW
SCALE: 3/4" = 1'-0"



TRASH RAKE DETAIL
SCALE: 3/4" = 1'-0"
(3 REQUIRED)



FLAP GATE DETAIL
SCALE: 3/4" = 1'-0"

NOTES:
1. NEW BOX TO BE FDOT "J" BOTTOM INDEX 425-010.



CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

CIVIL / STRUCTURAL / MEP ENGINEER:
R.J.Behar & Company, Inc.
Engineers-Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

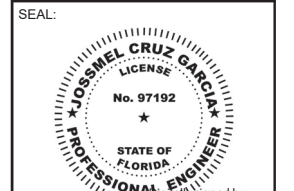
OWNER:
CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166



**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

PHASE:
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Designed by
Jossmel Cruz Garcia
Date: 2023.11.03
08:25:46 p.m.

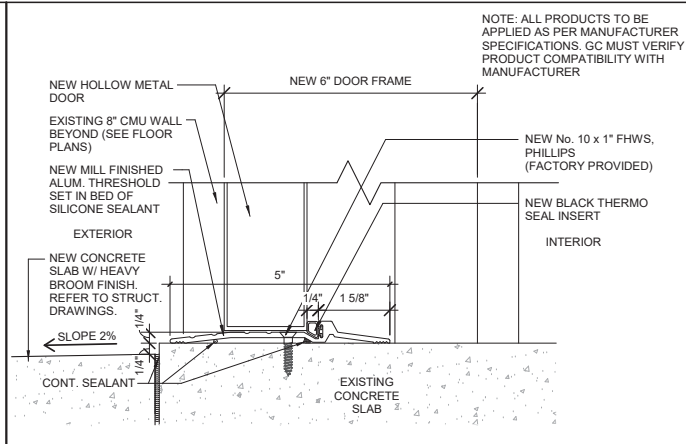
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No.	DESCRIPTION	DATE

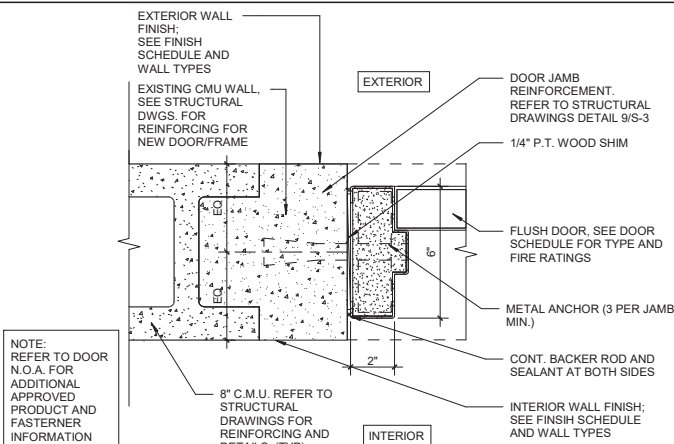
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Project No: 22060
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Drawn: AS
Checked: JCG

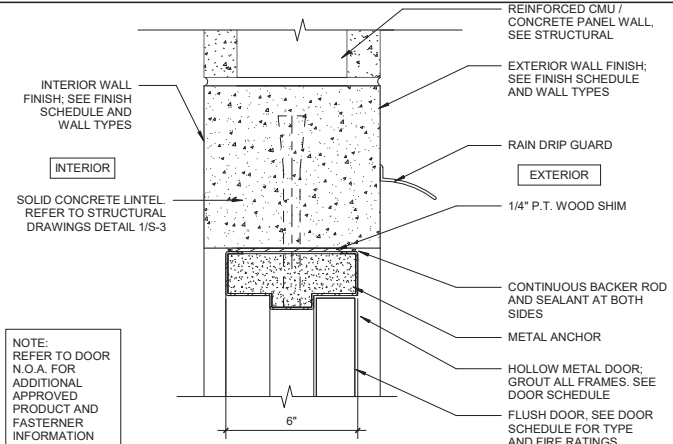
SHEET:
C-2



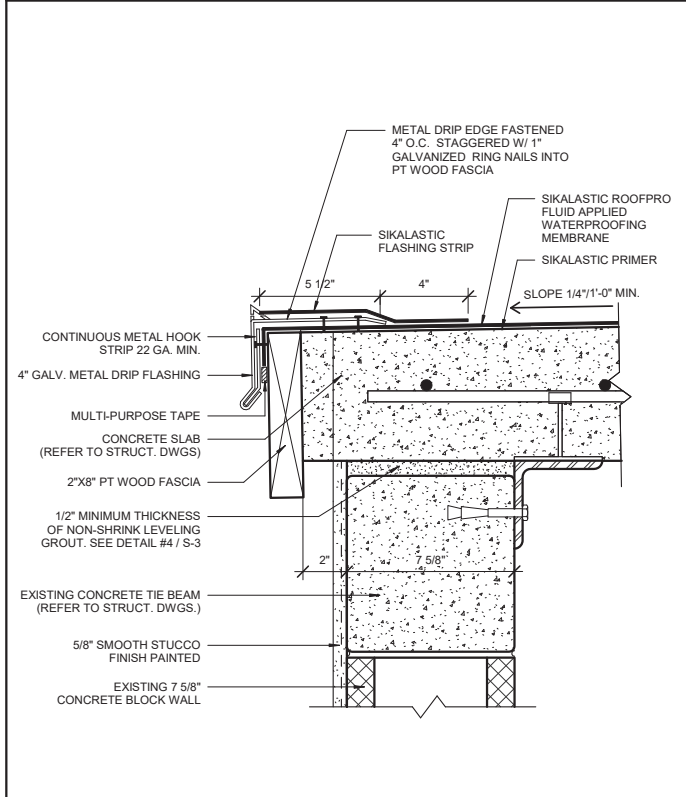
3 THRESHOLD DETAIL
A800 6" = 1'-0"



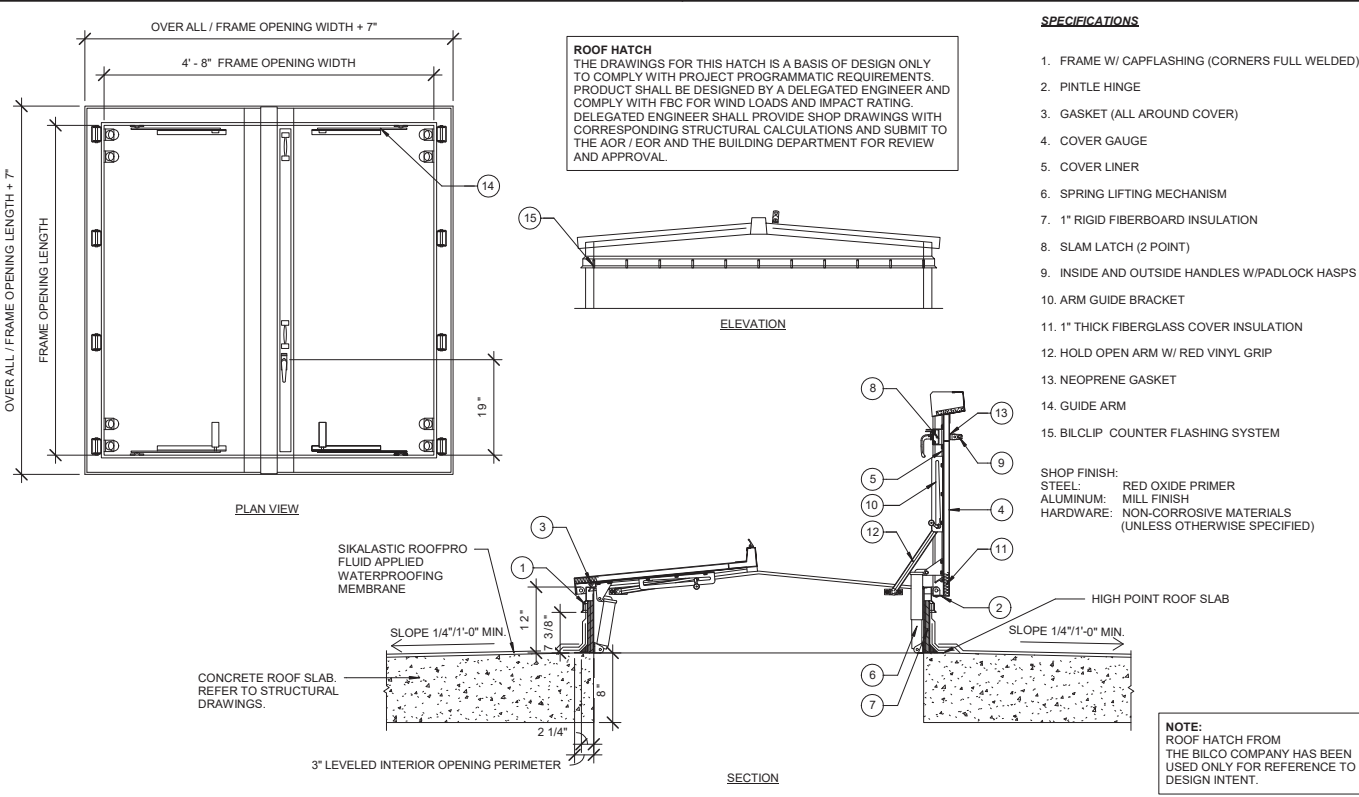
2 DOOR JAMB DETAIL
A800 3" = 1'-0"



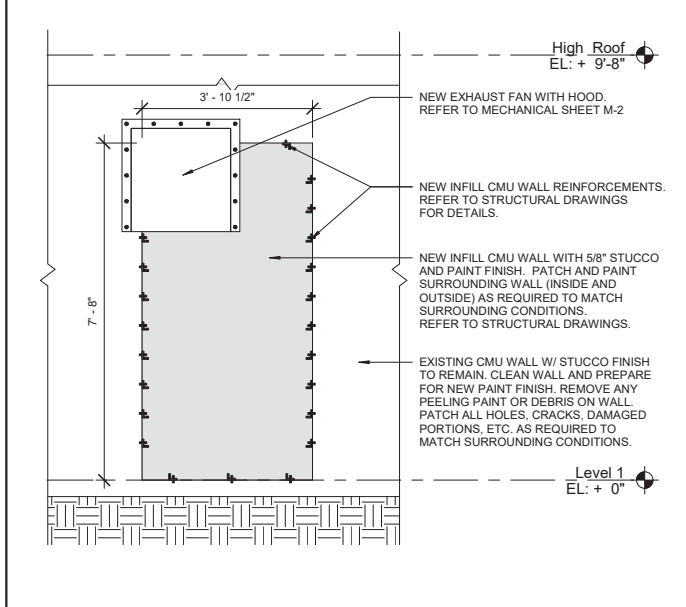
1 DOOR JAMB DETAIL
A800 3" = 1'-0"



4 ROOF FASCIA DETAIL
A800 3" = 1'-0"



5 ROOF HATCH
A800 1/16\"/>



6 INFILL CMU WALL
A800 1/2\"/>

DOOR SCHEDULE

DOOR NUMBER	ROOM	DOOR			FRAME				FIRE RATING	HARDWARE	REMARKS				
		WIDTH	HEIGHT	THICK	TYPE	MAT'L	GLASS	TYPE				MAT'L	HEAD	JAMB	THRESHOLD
100	WATER PUMP STATION	3'-0"	7'-0"	1 3/4"	A	HM	N/A	36x84 F	HM-1	2/A800	3/A800	4/A800	90	SET 1	

DOOR AND FRAME GENERAL NOTES

- LOCATE DOOR FRAMES WITH HINGE JAMB 4" FROM THE CORNER OF THE SPACE UNLESS SHOWN CENTERED IN THE WALL OR DIMENSIONED OTHERWISE.
- LABELS ARE ASTM CLASSIFICATIONS WITH FIRE RATINGS IN MINUTES. FIRE RATINGS OR LABELS REQUIRED FOR DOOR APPLY EQUALLY TO THE FRAME AND HARDWARE.
- ALL THRESHOLDS SHALL CONFORM TO THE MOST STRINGENT REQUIREMENTS OF THE AMERICANS WITH DISABILITIES ACT (ADA) AND ALL OTHER ACCESSIBILITY CODES ENFORCED BY THE AUTHORITIES HAVING JURISDICTION.

DOOR AND FRAME:
36" X 84" - 16 GAUGE OUT-SWING, SINGLE, FLUSH COMMERCIAL DOOR AND FRAME. LARGE AND SMALL MISSILE IMPACT RATED. PRESSURES - +70 PSF. PROVIDE MIAMI DADE COUNTY VALID NOTICE OF ACCEPTANCE #21-0504.07 OR EQUAL.

HARDWARE SET 1

HINGES:
IVES 30B1HW 3 KNUCKLE, CONCEALED BEARING, HEAVY WEIGHT FULL MORTISE HINGE, 304 SERIES STAINLESS STEEL WITH NON-REMOVABLE PIN (OR EQUAL).

KEY LOCK:
ALLEGION SCHLAGE (OR EQUAL) CYLINDER KEY LOCK TO MATCH EXISTING. KEY TO CITY'S MASTER KEY SYSTEM.

CLOSER:
LCN 4040XP SERIES HEAVY DUTY DOOR CLOSER (OR EQUAL).

KICK PLATE:
IVES 8400 ARMOR KICK PLATE .050" 17" HIGH X 34" WIDE. FINISH - SATIN STAINLESS STEEL BOTH SIDES (OR EQUAL).

PULL PLATE:
IVES 8303 PULL PLATE HEAVY DUTY 1" ROUND PULL 4"x16" SATIN STAINLESS STEEL 630 (OR EQUAL).

PUSH PLATE:
IVES 8300 PUSH PLATE HEAVY DUTY 4"x16" SATIN STAINLESS STEEL 630 (OR EQUAL).

THRESHOLD:
ALUMINUM THRESHOLD AS REQUIRED PER N.O.A.

ABBREVIATIONS
MAT'L MATERIAL
HM HOLLOW METAL

FIRE LABEL DESIGNATION
"B" LABEL - 90 MINUTE FIRE RATED

FINISHES
A - PAINT HM FRAMES
B - PAINT HM DOOR

DOOR TYPE	FRAME TYPE
SEE SCHEDULE	SEE SCHEDULE
TYPE A	SINGLE DOOR
HOLLOW METAL SWINGING DOOR	HM-1
SINGLE/FLUSH	

BA

CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

STRUCTURAL / MEP ENGINEER:
R.J. Behar & Company, Inc.
Engineers - Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

OWNER:
CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166

**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

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SEAL:

ON BEHALF OF BERMELO AJAMIL & PARTNERS, INC.

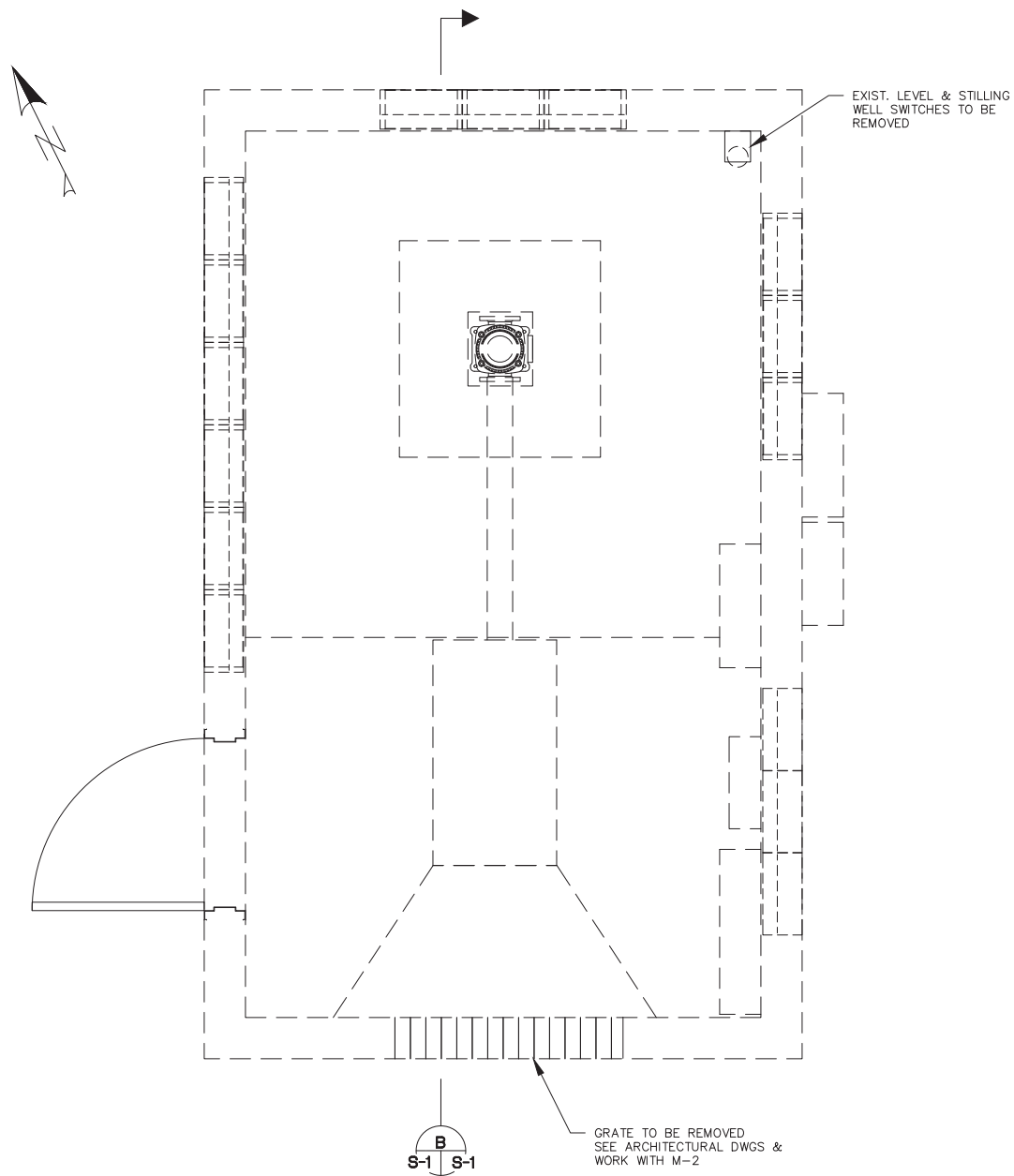
REVISIONS:

No.:	DESCRIPTION	DATE:

TITLE:
**DOOR, WALL, ROOF
DETAILS AND
SCHEDULES**

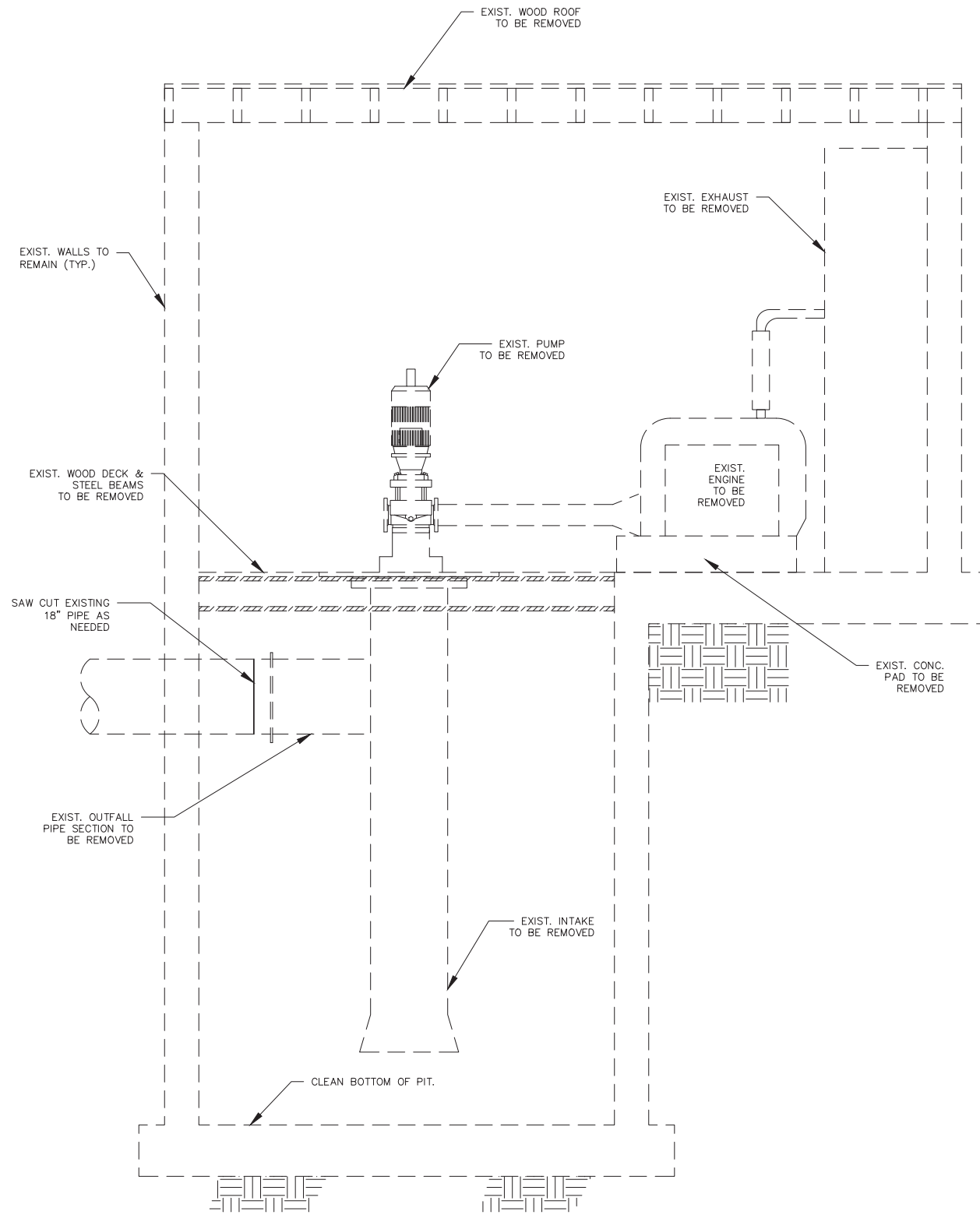
Project No: 02296.000
Date: 8/4/2023
Scale: As indicated
Format: 24" x 36"
Drawn: Author
Checked: Checker

SHEET:
A800



PUMP STATION PLAN VIEW
SCALE: 3/4"=1'-0"

- NOTES:
1. SEE DEMOLITION PHOTOS SHEET S-2



PUMP STATION SECTION **B**
SCALE: 3/4"=1'-0" **S-1 S-1**



CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

CIVIL / STRUCTURAL / MEP ENGINEER:
R.J.Behar & Company, Inc.
Engineers • Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

OWNER:
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201 WESTWARD DR
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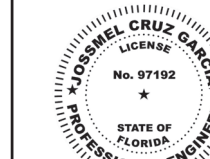


**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

PHASE:
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Jossmei Cruz Garcia
Jossmei Cruz Garcia
Date: 2023/11/15
08:24:30 -0400

REVISIONS:

No.:	DESCRIPTION	DATE:

TITLE:
**STRUCTURAL &
MECHANICAL
DEMOLITION**

Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn: AS
Checked: JCG

SHEET:
S-1



DEMOLITION KEY NOTES:		
MARK	DESCRIPTION	QTY.
1	EXISTING PUMP AND MOTOR	1
2	ENGINE DRIVE	1
3	WOOD & STEEL BEAM DECK	1
4	EXHAUST AND EXHAUST DISTRIBUTION COVER	1
5	STATION ROOF AND ACCESS HATCH	1
6	CONCRETE INFLOW BOX	1
7	LEVEL SWITCHES & STILLING WELL	1



ELECTRICAL SYSTEM TO BE MODIFIED. SEE ELECTRICAL DRAWINGS.

BA

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Bermello Ajamil & Partners
 4711 South LeJeune Road
 Coral Gables, FL 33146
 P: 305.859.2050

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R.J.Behar & Company, Inc.
 Engineers • Planners
 6861 SW 196th Ave suite 302,
 Fort Lauderdale, FL 33332
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CITY OF MIAMI SPRINGS
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**MIAMI SPRINGS
 HOOK SQUARE
 PUMP STATION**
 25 S. HOOK SQUARE
 MIAMI SPRINGS, FL 33166

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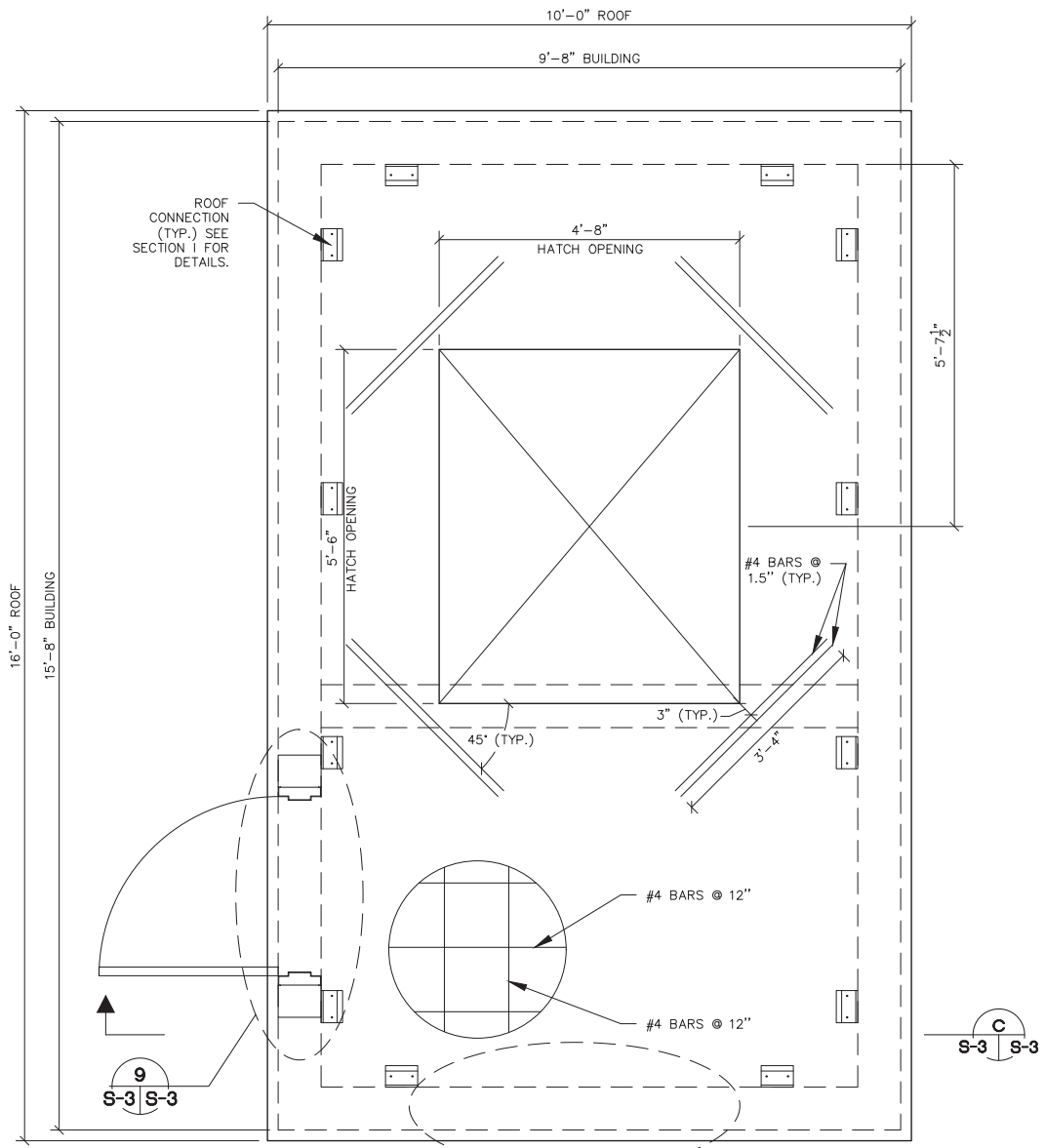
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No.:	DESCRIPTION	DATE:

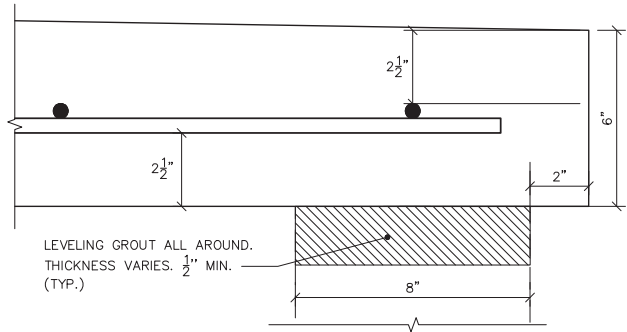
TITLE:
DEMOLITION PHOTOS

Project No: 22060
 Date: 10/11/2023
 Scale: N.T.S.
 Format: 24" x 36"
 Drawn: AS
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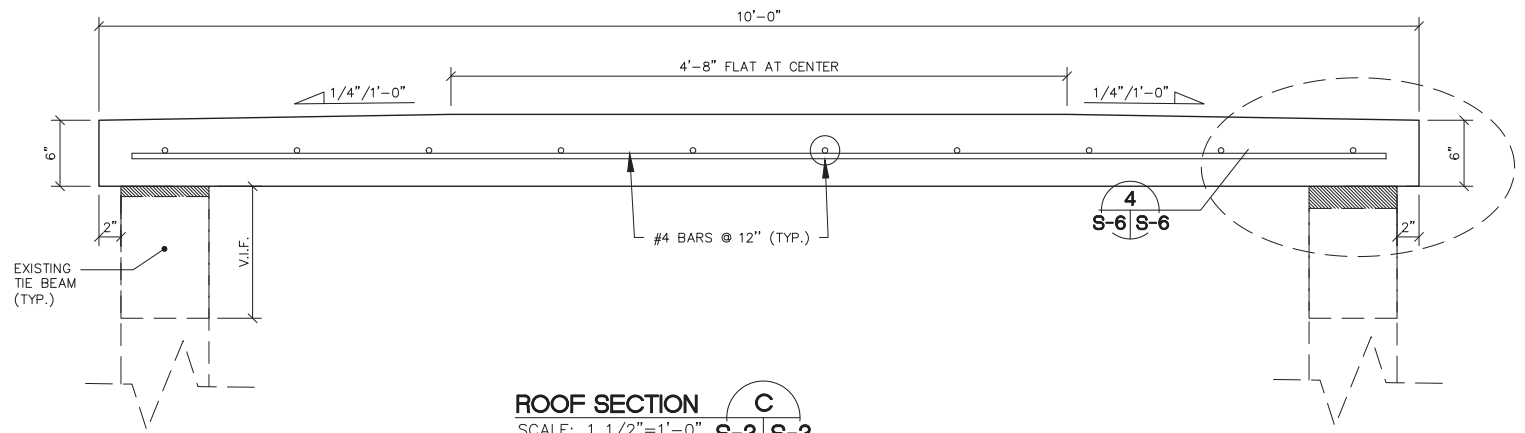
SHEET:
S-2



ROOF PLAN VIEW
SCALE: 3/4" = 1'-0"

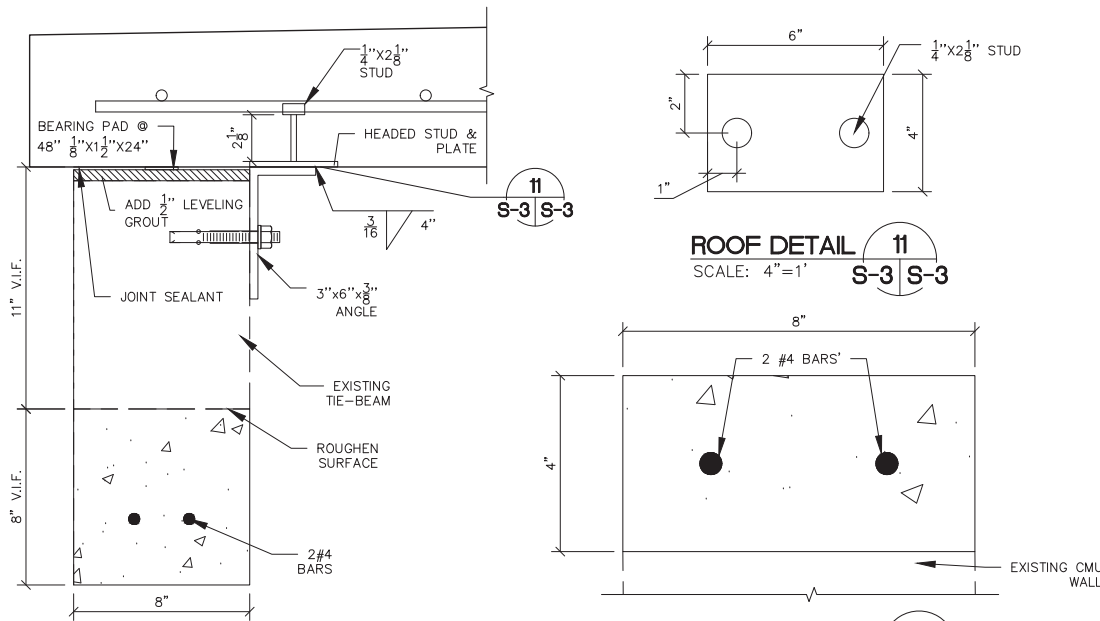


ROOF DETAIL 4
SCALE: 4" = 1'
S-3/S-3



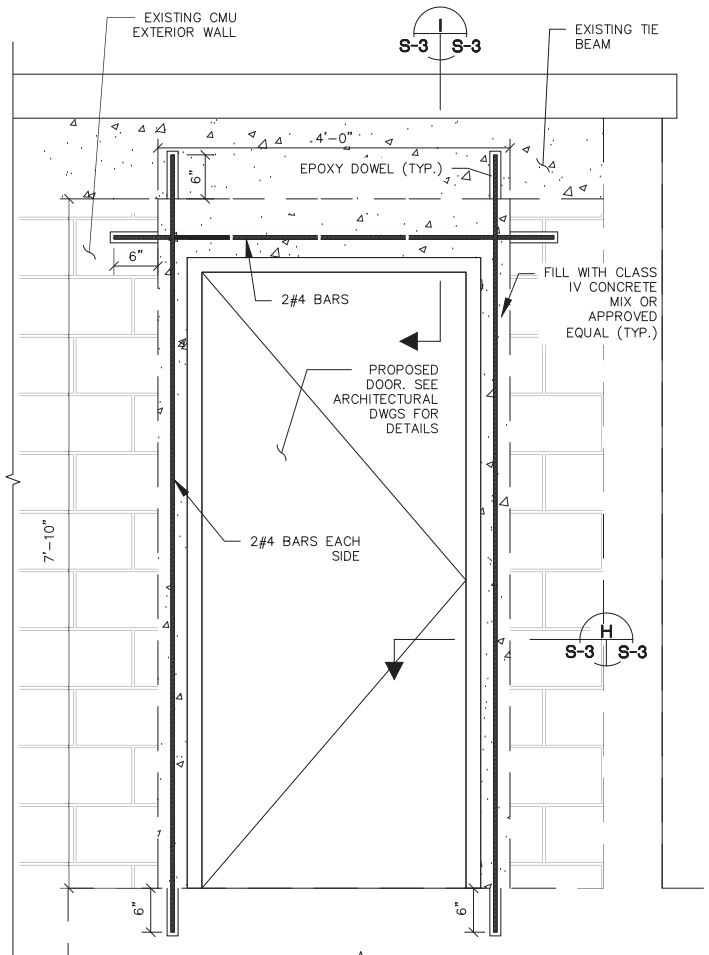
ROOF SECTION C
SCALE: 1 1/2" = 1'-0"
S-3/S-3

- GENERAL NOTES FOR CONCRETE:**
- CONCRETE DESIGN AND BAR DETAIL, WORKMANSHIP, AND GENERAL PROCEDURE OF CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 318, ACI 315, AND ACI 301, LATEST EDITION, UNLESS OTHERWISE NOTED IN SPECIFICATIONS.
 - MINIMUM CONCRETE STRENGTH AFTER 28 DAYS, UNLESS OTHERWISE NOTED:
 - CAST IN PLACE: 5,500 PSI (CLASS IV CONCRETE).
 - PRECAST CONCRETE: 5,500 PSI (CLASS IV CONCRETE).
CONCRETE SHALL BE ALLOWED TO ACHIEVE ITS MINIMUM CONCRETE COMPRESSIVE STRENGTH AFTER 28 DAYS PRIOR TO LOADING UNLESS APPROVED BY THE ENGINEER.
 - ALL STEEL REINFORCING SHALL BE A MINIMUM YIELD STRENGTH OF 60,000 PSI (F_y = 60,000 PSI) AS PER A.S.T.M. A615 GRADE 60 DEFORMED BILLET STEEL BAR, UNLESS OTHERWISE NOTED. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL THE NECESSARY CHAIRS, REBAR, TIES, SPACERS, ETC. TO SECURE AND SUPPORT THE REINFORCEMENT WHILE PLACING THE CONCRETE.
 - THE CONTRACTOR MAY SUBSTITUTE A PRECAST SLAB WITH SIMILAR REINFORCEMENT AS PER MEANS AND METHOD. SUBMIT DETAILS OF INSTALLATION, INCLUDING ANY EMBEDDED HOOKS REQUIRED TO PLACE THE SLAB, AND PROVIDE A SHEAR KEY AT WALL TO SLAB INTERFACE.
 - CONCRETE COVER = 2 1/2" (+1/2", -0")
 - STRUCTURE SHALL BE DESIGNED USING THE STRUCTURAL DESIGN CRITERIA ON THIS SHEET.
 - FOR ROOF SEALING COAT SEE ARCHITECTURAL PLANS.
 - CONFIRM LOCATION OF EXISTING REINFORCEMENT PRIOR TO DRILLING HOLES FOR EPOXY DOWELS AT DOORWAY.
 - PROVIDE TEMPORARY BRACING SUPPORT AT DOORWAY.

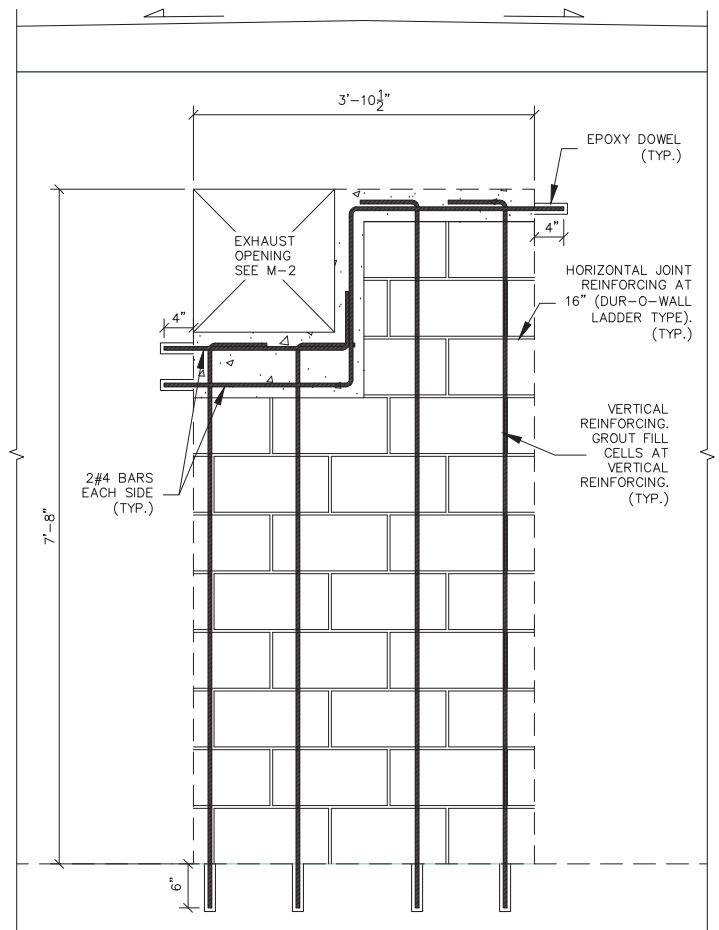


DOOR FASCIA SECTION I
SCALE: 3" = 1'
S-3/S-3

DOOR JAMB SECTION H
SCALE: 4" = 1'
S-3/S-3



DOOR CONCRETE FRAME DETAIL 9
SCALE: 1" = 1'
S-3/S-3



INFILL CMU WALL DETAIL 10
SCALE: 1" = 1'
S-3/S-3



CIVIL ENGINEER / ARCHITECT:
Bermello Ajami & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

CIVIL / STRUCTURAL / MEP ENGINEER:
R.J.Behar & Company, Inc.
Engineers-Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

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MIAMI SPRINGS, FL 33166



**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

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No. 97192
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Signed by Josmel Cruz Garcia
Date: 2023.11.03
08:29:03 -0400'

REVISIONS:

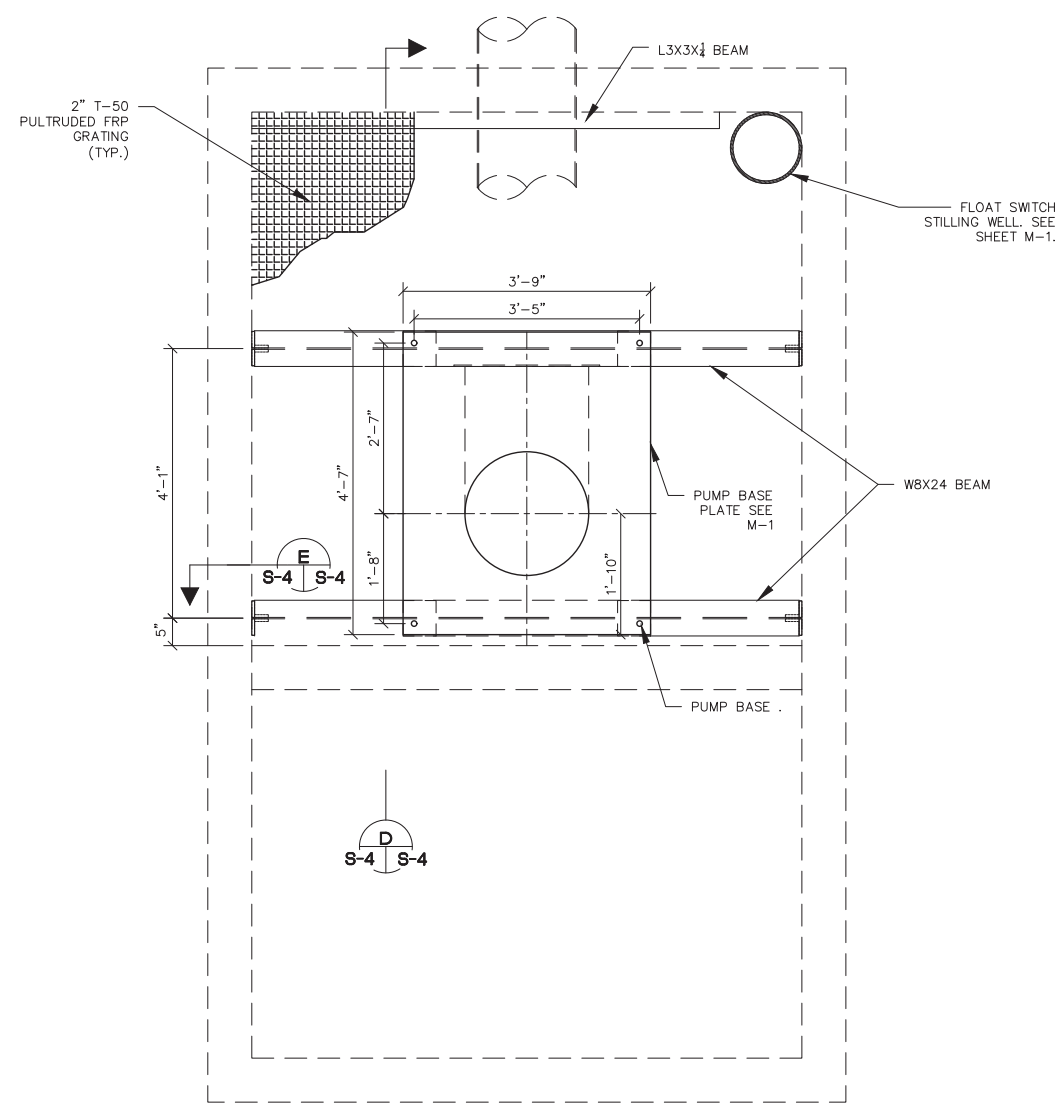
No.:	DESCRIPTION	DATE:

TITLE:
ROOF DETAILS

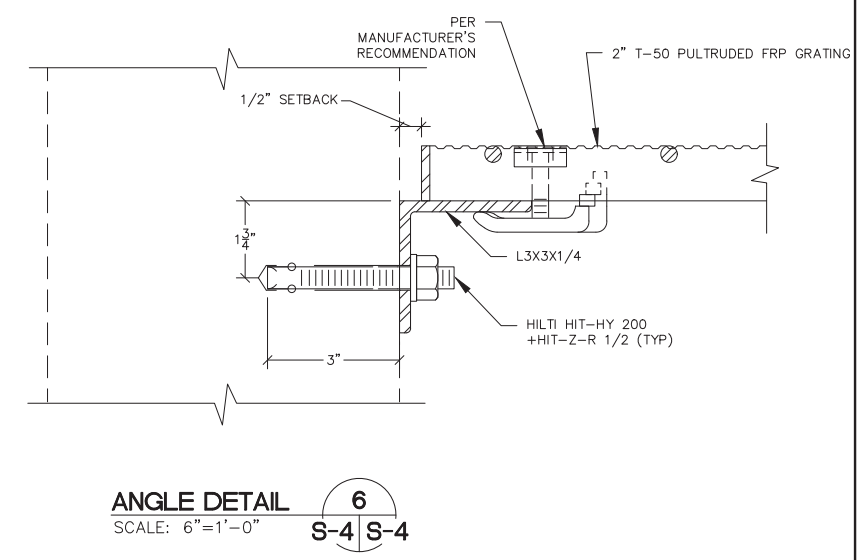
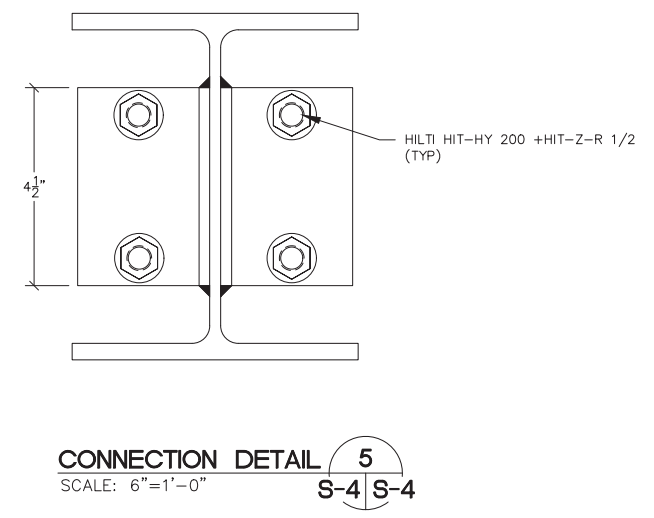
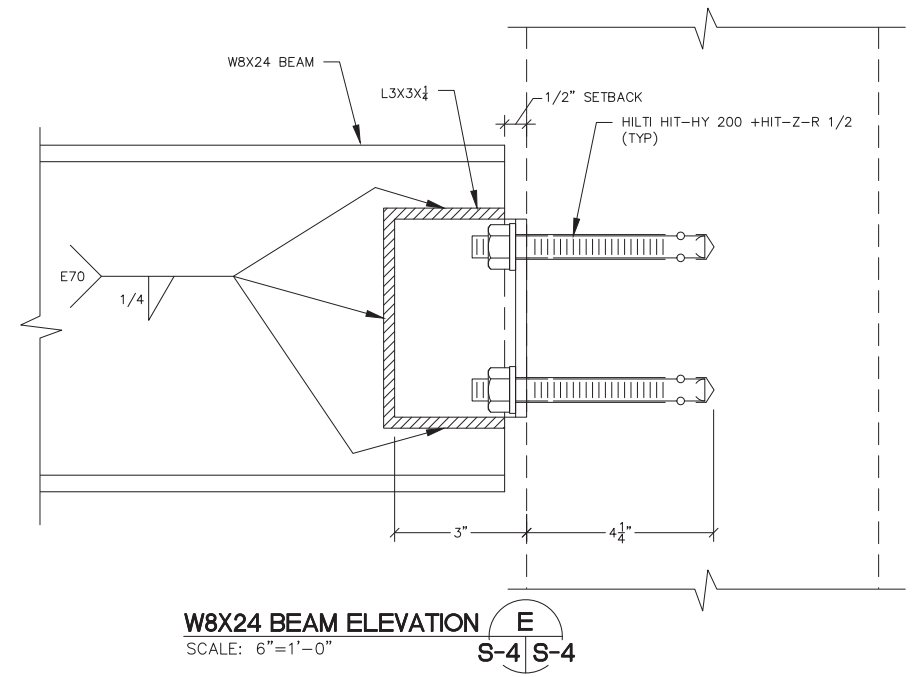
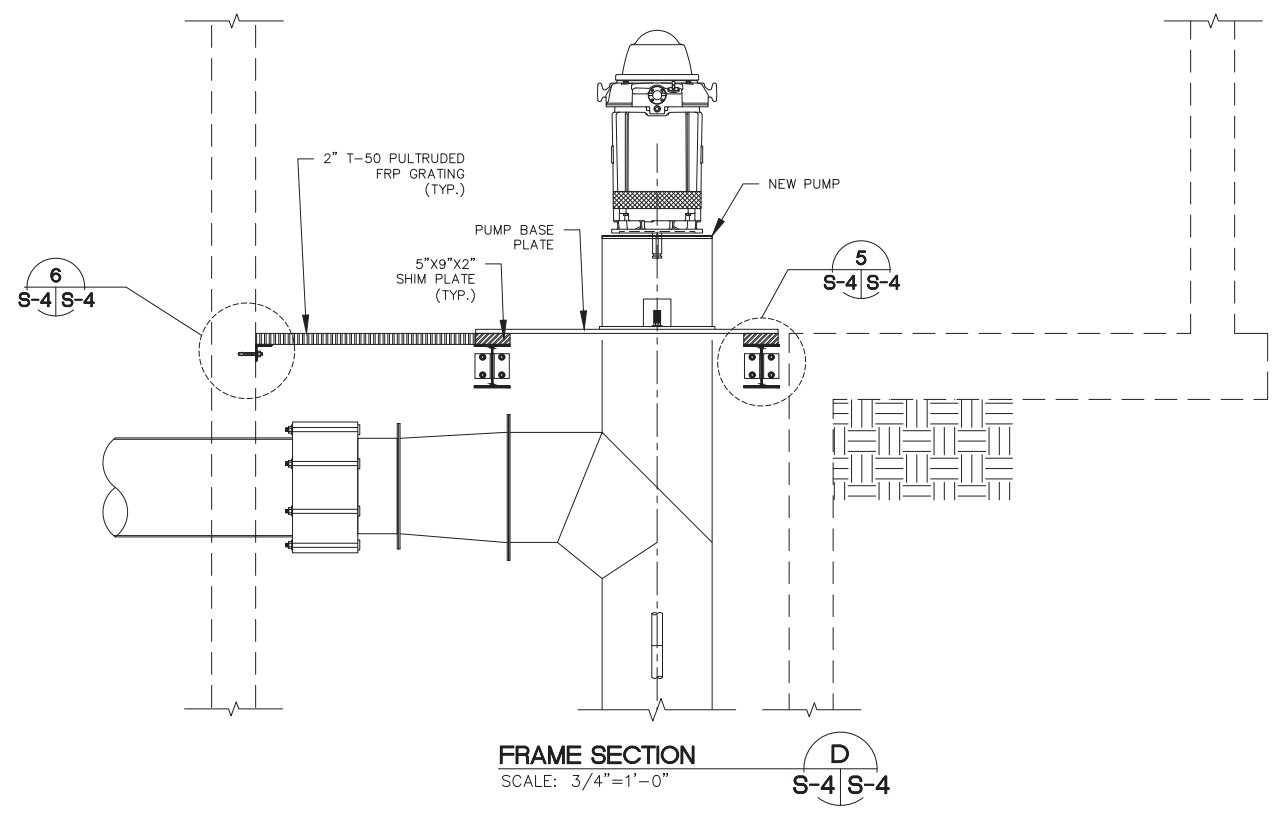
Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn: AS
Checked: JCG
SHEET:

S-3

NOTES:
1. V.I.F. = VERIFY IN FIELD.



PUMP FRAMING PLAN
SCALE: 3/4" = 1'-0"



BA

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Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

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R.J. Behar & Company, Inc.
Engineers - Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

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MIAMI SPRINGS

**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

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Garcia
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No.:	DESCRIPTION	DATE:

TITLE:
FRAMING DETAILS

Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn: AS
Checked: JCG

SHEET:
S-4



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Bermello Ajamil & Partners
 4711 South LeJeune Road
 Coral Gables, FL 33146
 P: 305.859.2050

CIVIL / STRUCTURAL / MEP ENGINEER:
R.J. Behar & Company, Inc.
 Engineers - Planners
 6861 SW 196th Ave suite 302,
 Fort Lauderdale, FL 33332
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CITY OF MIAMI SPRINGS
 201 WESTWARD DR
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**MIAMI SPRINGS
 HOOK SQUARE
 PUMP STATION**
 25 S. HOOK SQUARE
 MIAMI SPRINGS, FL 33166

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No.:	DESCRIPTION	DATE:

TITLE:
**PUMP INSTALLATION
 DETAILS**

Project No: 22060
 Date: 10/11/2023
 Scale: N.T.S.
 Format: 24" x 36"
 Drawn: AS
 Checked: JCG

SHEET:
M-1

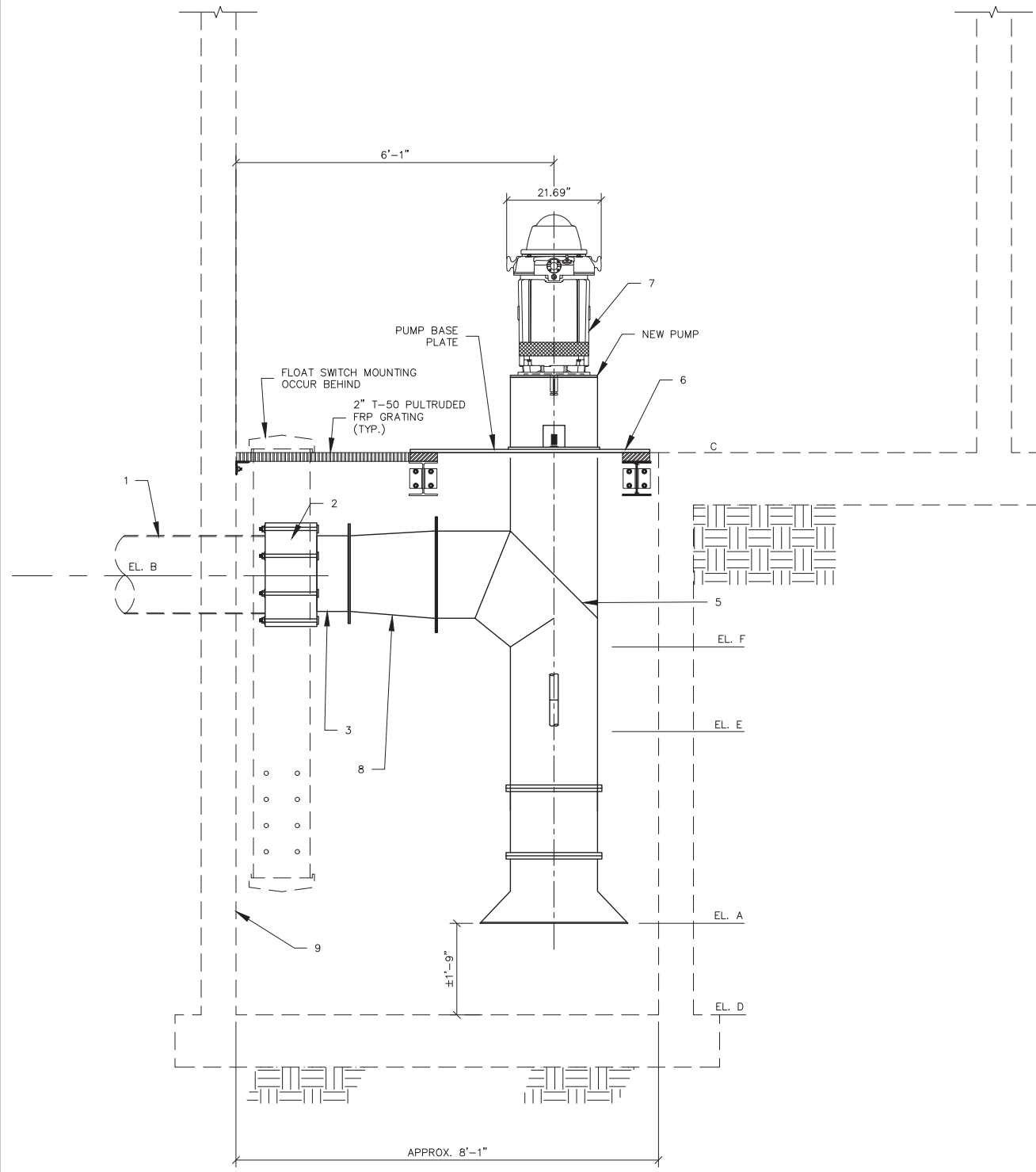
KEY NOTES:

MARK	DESCRIPTION	DETAIL	SPEC	REMARKS
1	EXIST. 18" DISCHARGE PIPE			CAST IRON OR STEEL
2	DRESSER COUPLING			CONFIRM SIZE BEFORE ORDERING
3	18" DI NIPPLE			PLAIN END - FLANGED END, SIZE TO FIT
4	18" TO 20" DI FLANGED REDUCER			
5	AXIAL FLOW PUMP			20" NATIONAL PUMP CO. #LL20P, SUPPLIED BY OTHERS
6	BASE PLATE			SUPPLIED BY OTHERS
7	ELECTRIC MOTOR FOR PUMP			NIDEC MOTOR CORP MODEL 09-3422, SUPPLIED BY OTHERS
8	20' AWWA A50# FLANGED DISCHARGE			SUPPLIED BY OTHERS
9	EXISTING WET WELL TO REMAIN			

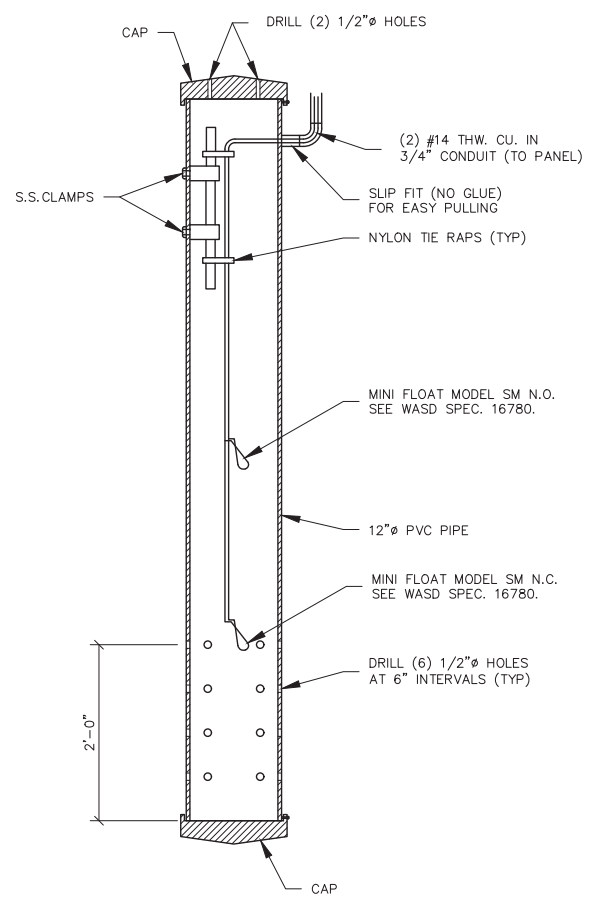
ELEVATION SCHEDULE (NGVD 29)

MARK	ELEVATION
A	-1.60'
B	TO MATCH EXISTING
C	7.40'
D	-3.35'
E	2.57'
F	5.0'*

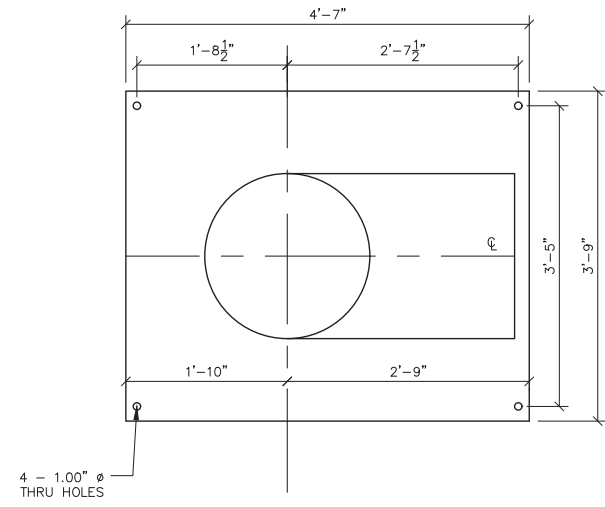
* TO BE CONFIRMED BY CITY OF MIAMI SPRINGS.



PUMP STATION ELEVATION WEST
 SCALE: 3/4" = 1'-0"

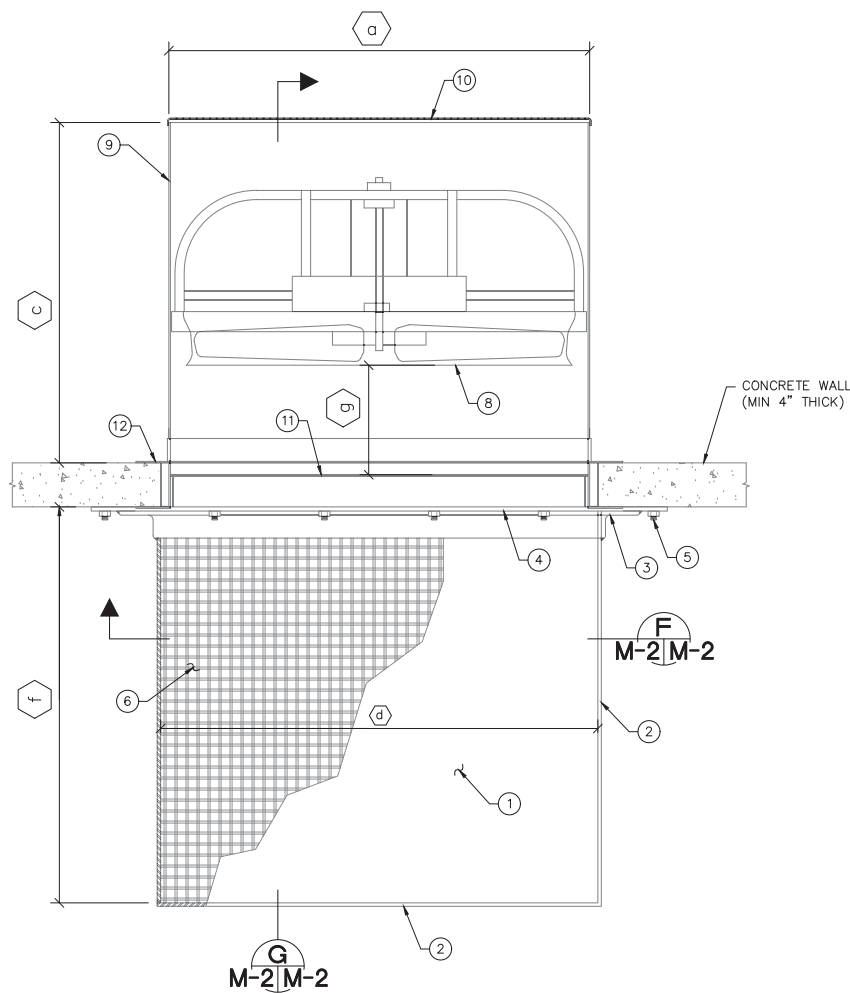


FLOAT SWITCH MOUNTING DETAILS
 SCALE: NOT TO SCALE

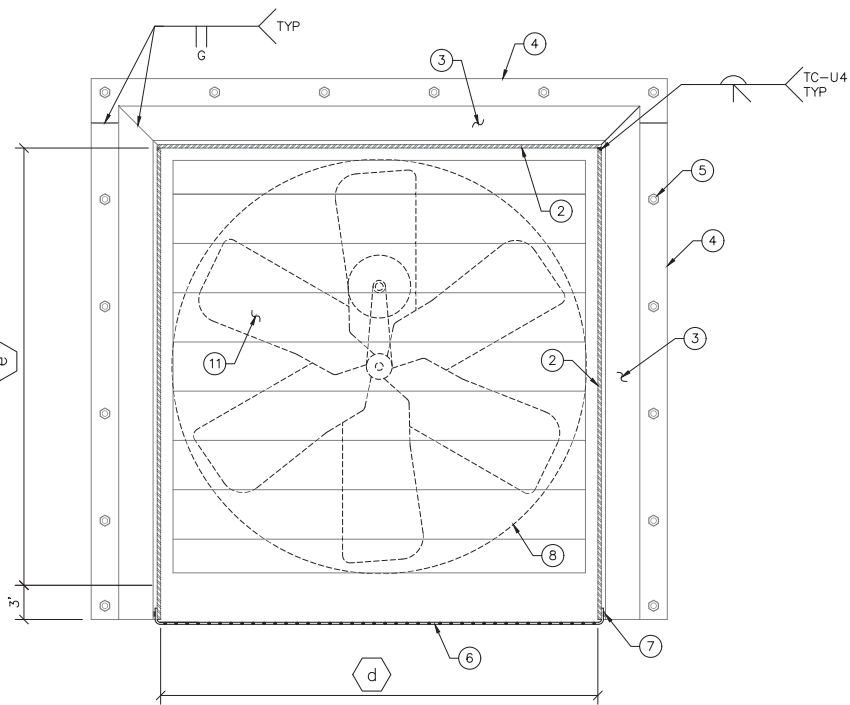


BASE PLATE DETAIL '7'
 SCALE: 1" = 1'-0"
 * TO BE CONFIRMED BY PUMP MANUFACTURER

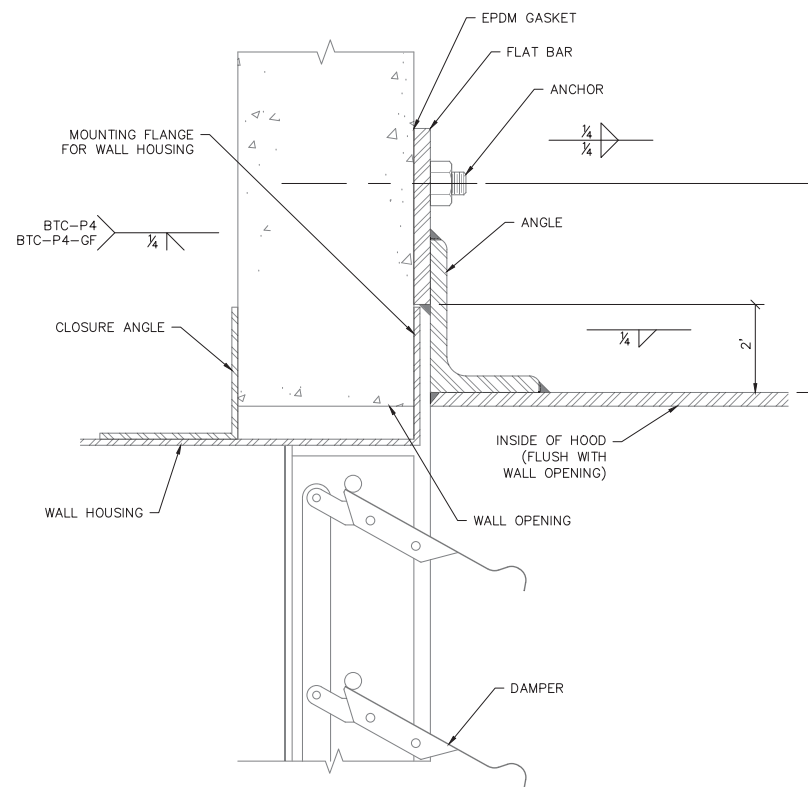
NOTES:
 1. PUMP & MOTOR TO BE PROVIDED BY OWNER FOR CONTRACTOR TO INSTALL.



EXHAUST FAN HOOD PLAN
SCALE: 1-1/2" = 1'-0"

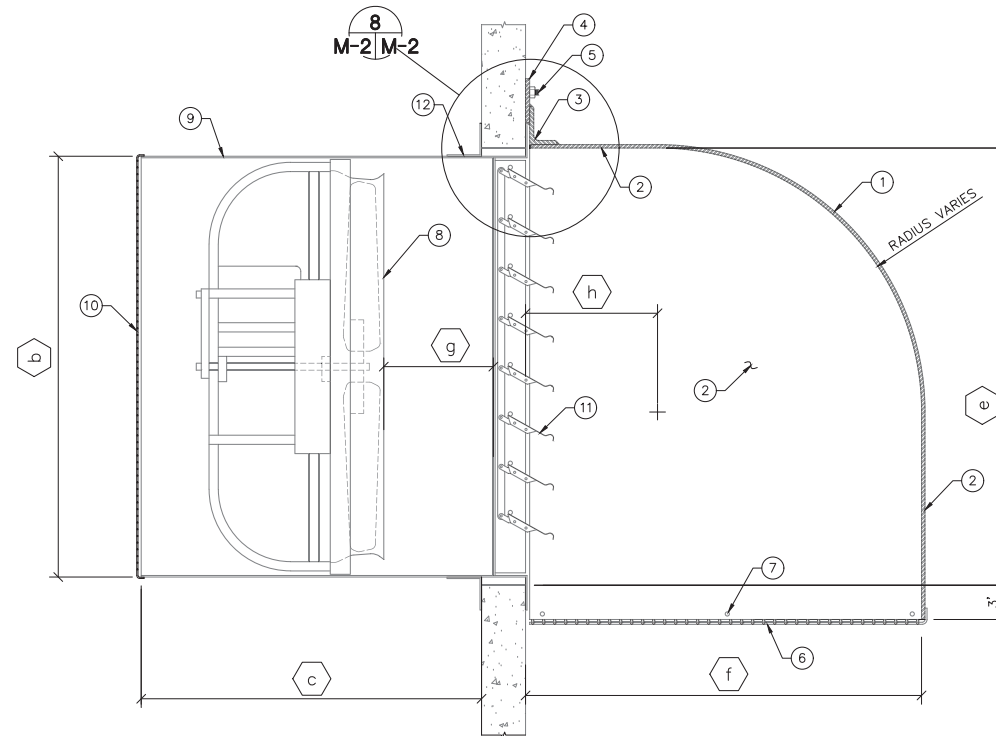


SECTION
SCALE: 1-1/2" = 1'-0"



TYPICAL TOP AND SIDES

DETAIL
SCALE: 6" = 1'-0"



SECTION
SCALE: 1-1/2" = 1'-0"

KEY NOTES		
MARK	DESCRIPTION	REMARKS
①	HOOD: 5/16" BENT PLATE	
②	HOOD: 5/16" BENT PLATE	
③	FRAME: L 3/2x2 1/2x3/8	
④	3/8"x4" SQUARE BACK PLATE	
⑤	ANCHOR: 1/2" DIA EPOXY ADHESIVE, SPACED 9" TO 12" ON CENTER	THREADED ROD, NUT AND WASHER, HILTI HIT-RE 500, 2 3/4" EMBEDMENT
⑥	BIRD SCREEN, 316 SS	11 GAUGE, OSHA COMPLIANT FAN GUARD
⑦	1/4" DIAMETER SELF-TAPPING MACHINE SCREW	Ø 1'-6" MAX ON CENTER
⑧	EXHAUST FAN, BELT-DRIVEN	GREENHECK'S LEVEL 1, 12" DIRECT DRIVE EXHAUST FAN, MODEL S1-12-432-D
⑨	WALL HOUSING, GALVANIZED STEEL	18 GAUGE
⑩	WELDED STEEL WIRE OSHA COMPLIANT FAN GUARD	COAT GUARD WITH THERMAL SETTING POLYESTER URETHANE
⑪	EXHAUST DAMPER, GRAVITY OPERATED	GALVANIZED FRAME, ALUMINUM BLADES, VINYL BLADE SEALS, PULL CHAIN LINKAGE FOR MANUAL OPER
⑫	CLOSURE ANGLE	

- (1) ALL MATERIALS 304L STAINLESS STEEL, UNLESS OTHERWISE NOTED
(2) THREADED RODS, NUTS, WASHERS AND SCREWS: 316 STAINLESS STEEL

DIMENSION SCHEDULE		
MARK	DESCRIPTION	REMARKS
a	WALL HOUSING OUTSIDE WIDTH	SEE TABLE BELOW
b	WALL HOUSING OUTSIDE HEIGHT	= WALL HOUSING OUTSIDE WIDTH
c	WALL HOUSING PROJECTION FROM WALL	SEE TABLE BELOW
d	WALL OPENING WIDTH	SEE TABLE BELOW
e	WALL OPENING HEIGHT	= WALL OPENING WIDTH
f	HOOD PROJECTION FROM WALL (INSIDE MEASUREMENT)	= WALL HOUSING OUTSIDE WIDTH
g	MINIMUM DISTANCE BETWEEN PROPELLER AND DAMPER	SEE TABLE BELOW
h	DISTANCE FROM CENTER OF HOOD RADIUS TO WALL	= 6" FOR FANS 18" AND SMALLER

FAN SIZE	a	c	d	g
12	18 3/4	23	19 1/4	7

ALL DIMENSIONS GIVEN IN INCHES AND BASED ON GREENHECK FANS.

GENERAL NOTES:

- INSIDE OF HOOD, TOP AND SIDES, SHALL BE FLUSH WITH WALL OPENING.
- EXHAUST FAN HOODS SHALL BE FABRICATED IN ACCORDANCE WITH SECTION 05550.
- MINIMUM CONNECTION OF ADJACENT STEEL PARTS SHALL BE MADE WITH FILLET OR FLARE BEVEL WELD ALL AROUND, UNLESS OTHERWISE NOTED.
- ALL EXPOSED EDGES OF WELDS AND ALL SHARP EDGES SHALL BE GROUND SMOOTH, WITH A 1/8-INCH MINIMUM RADIUS.
- EACH HOOD SHALL BE PROVIDED WITH PROPERLY DESIGNED LIFTING LUGS TO FACILITATE HANDLING AND INSTALLATION.
- EXHAUST FANS SHALL CONFORM TO SPECIFICATION SECTION 15510.
- DIMENSIONS ARE SOLELY FOR CONTRACTOR'S REFERENCE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS BASED ON EQUIPMENT SELECTED.
- CONTRACTOR MAY PROPOSE A NON-CURVED HOOD, SUBJECT TO DISTRICT REVIEW AND APPROVAL.
- SUBMIT SHOP DRAWINGS FOR FAN, FAN ACCESSORIES AND HOOD, IN ACCORDANCE WITH SECTION 01300.
- THERE SHALL BE ADEQUATE SPACE AROUND THE FAN HOUSING TO PERMIT MAINTENANCE AND REPLACEMENT OF THE FAN.
- SOUND RATING SHALL COMPLY WITH OSHA STANDARDS.
- PROVIDE TAMPER RESISTANT FASTENERS AT LOCATIONS DESIGNATED BY THE DISTRICT AS HIGH SECURITY (INTAKE AND EXHAUST).

DESIGN CRITERIA	
BASIC WIND SPEED V	= 180 MPH
WIND EXPOSURE	= C
RISK CATEGORY	= IV (ADJUSTED FOR 200-YR MRI)



CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

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R.J. Behar & Company, Inc.
Engineers-Planners
6861 SW 196th Ave suite 302,
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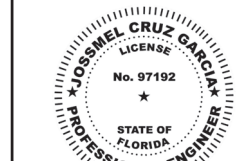


**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

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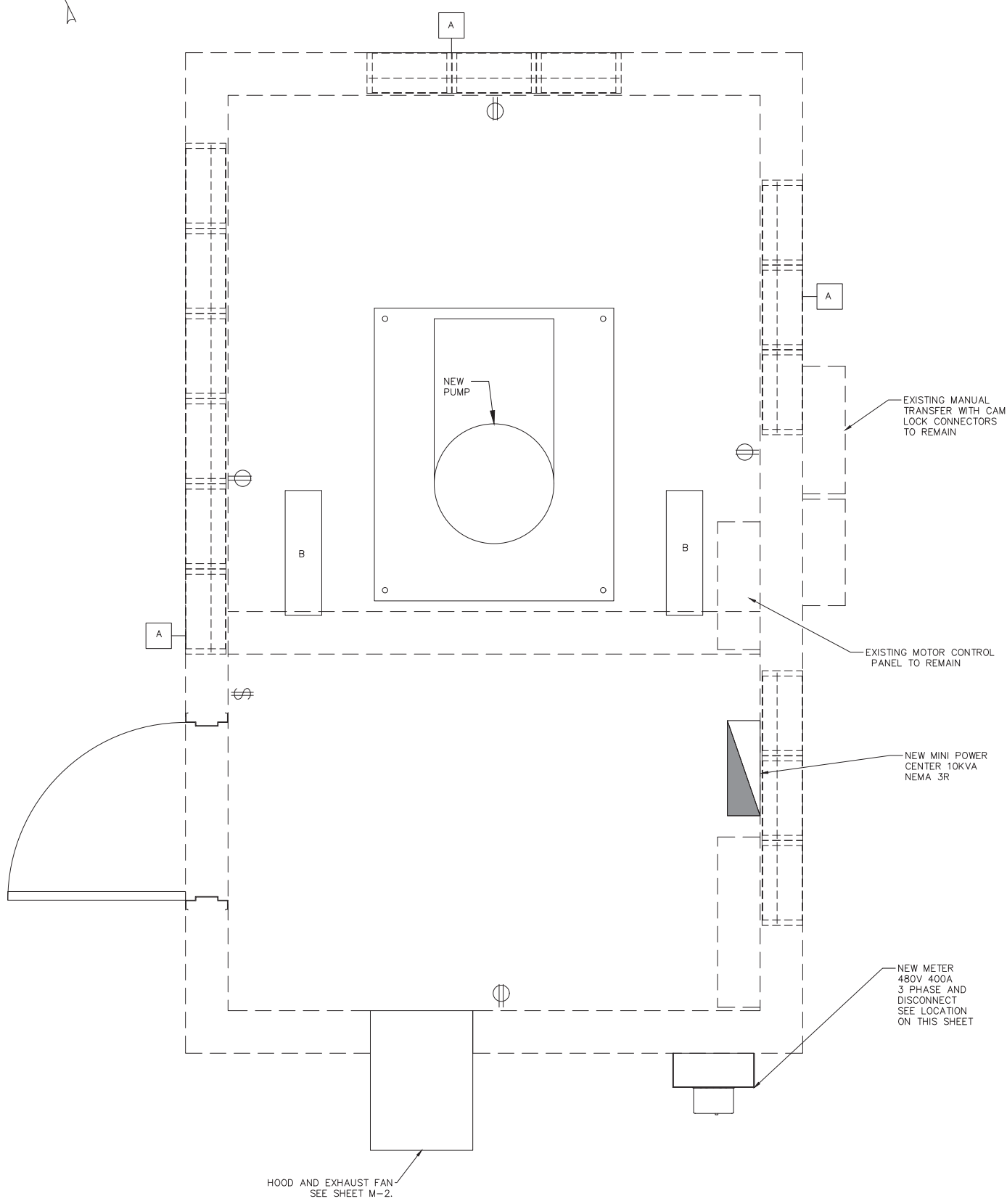
REVISIONS:

No.	DESCRIPTION	DATE:

TITLE:
**HOOD AND EXHAUST
FAN DETAILS**

Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn: AS
Checked: JCG

SHEET:
M-2



FLOOR PLAN
SCALE: 1" = 1'-0"

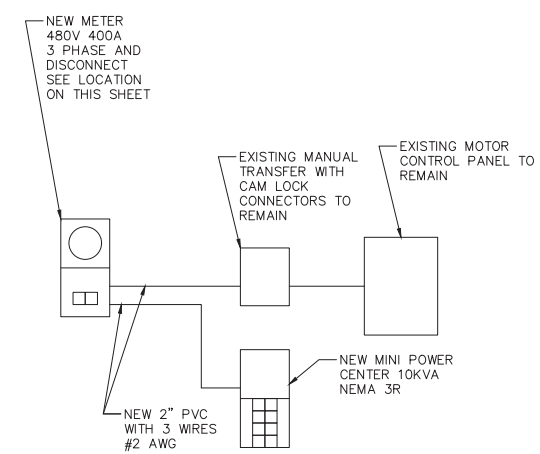
MINI POWER CENTER SCHEDULE "A"

TYPE : NEMA 3R 480V TO 120/240V MAINS : MAIN
MOUNTING : SURFACE 10 KVA BUS RATING : 60 AMP
LOCATION : SEE DRAWING VOLTAGE : 120/240V-1PHASE-3WIRE

DESCRIPTION	WIRE & COND. SIZE	LOAD (V.A.)	POLE TRIP	CKT. No.	A B		CKT. No.	POLE TRIP	LOAD (V.A.)	WIRE & COND. SIZE	DESCRIPTION
* LIGHTING A	#12-1"	950	1/20	1			2	1/20	1500	#12-1"	FAN MOTOR
LIGHTING B	#12-1"	950	1/20	3			4				
RECEPTACLES	#12-1"	1,800	1/20	5			6				
				7			8				
				9			10				
				11			12				
				13			14				
				15			16				
				17			18				
				19			20				
				21			22				
				23			24				
				25			26				
				27			28				
				29			30				

* CONTROLLED BY INDIVIDUAL PHOTOCELL

LOAD CALCULATION MINI POWER CENTER "A"
CONTINUOUS LOAD AT 125% = 3,400 VA X 1.25 = 4,250 VA
NON CONTINUOUS LOAD AT 100% = 1,800 VA
TOTAL DEMANDED LOAD = 6,050 VA
TOTAL DEMAND AMPS = 26 AMPS PER PHASE



RISER DIAGRAM
N.T.S.

- SCOPE:**
- 1-REMOVED THE EXISTING METER, ALL ELECTRIC CABLES, CONDUITS, RECEPTACLES AND ELECTRIC GEARS NOT TO BE USED FOR THIS PROJECT
 - 2-INSTALL METER.
 - 3-INSTALL NEW MINI POWER CENTER.
 - 4-INSTALL NEW OUTDOOR LIGHTING FIXTURES PER SCHEDULE.
 - 5-INSTALL NEW INDOOR LIGHTING FIXTURES PER SCHEDULE WITH MOTION SENSOR SWITCH.
 - 6-REWIRE NEW FURNISHED PUMP TO THE EXISTING MOTOR CONTROL PANEL.

LIGHTING FIXTURE SCHEDULE

TYPE	DESCRIPTION	MFR	FIXTURE CAT #	LAMPS	VOLT	MOUNTING	NOTES
A	FULL CUTOFF WALL PACK	XTRA LIGHT OR EQUAL	7500L-50K-SD-BZ-PC	3	120	WALL	SEE DETAILS FOR INSTALLATION. ANTI-VANDALISM.
B	VAPOR TIGHT LINEAR	XTRA LIGHT OR EQUAL	8 FEET-6000L-50K-SFA-HL	2	120	CEILING	SEE DETAILS FOR INSTALLATION. HAZARDOUS LOCATION



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4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

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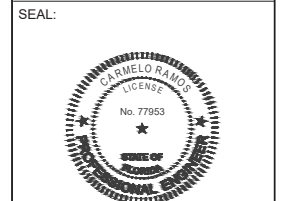
OWNER:
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**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

PHASE:
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Carmelo Ramos
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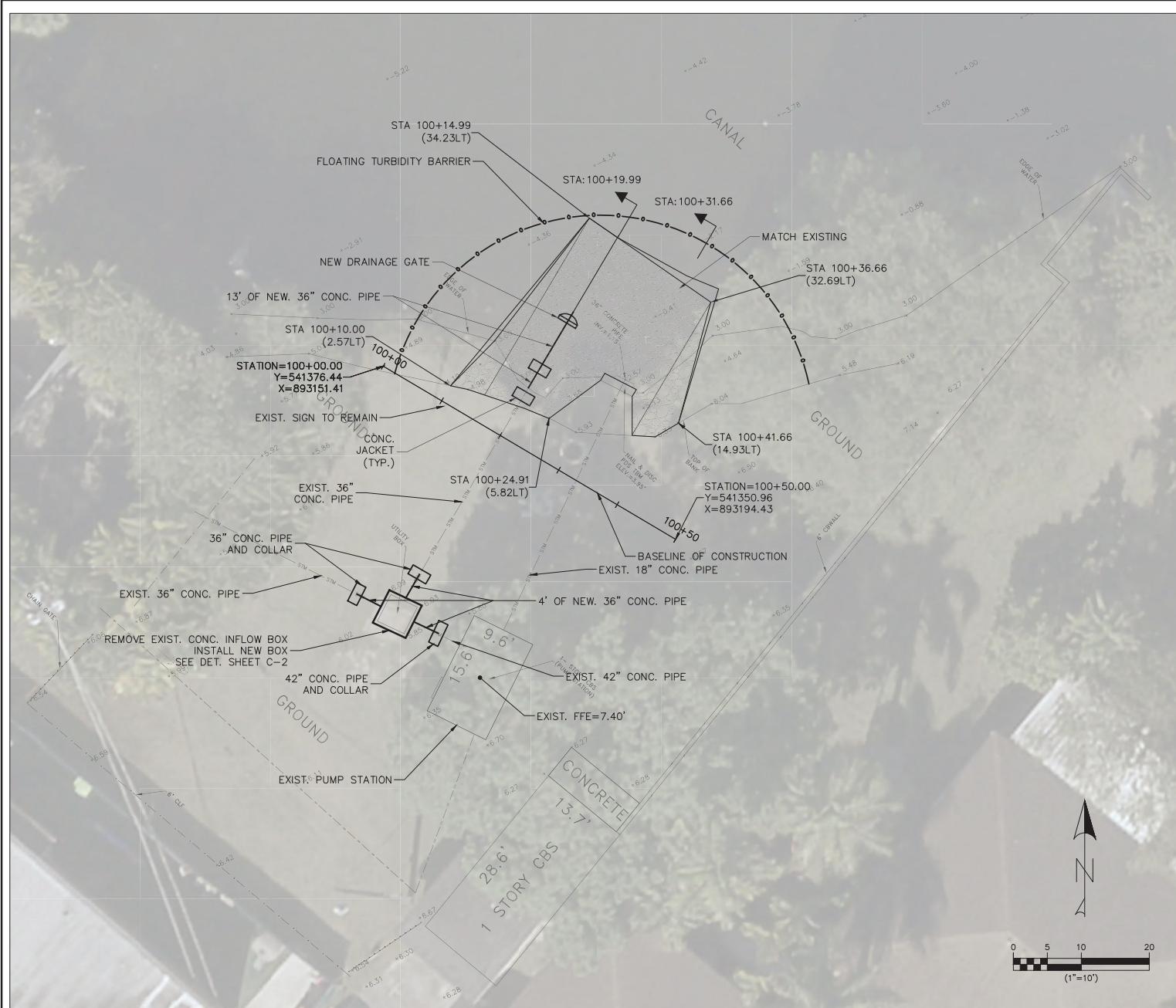
REVISIONS:

No.:	DESCRIPTION	DATE:

TITLE:
ELECTRICAL FLOOR PLAN

Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn:
Checked: CR

SHEET:
E-2



SITE PLAN VIEW
SCALE: 1" = 10'

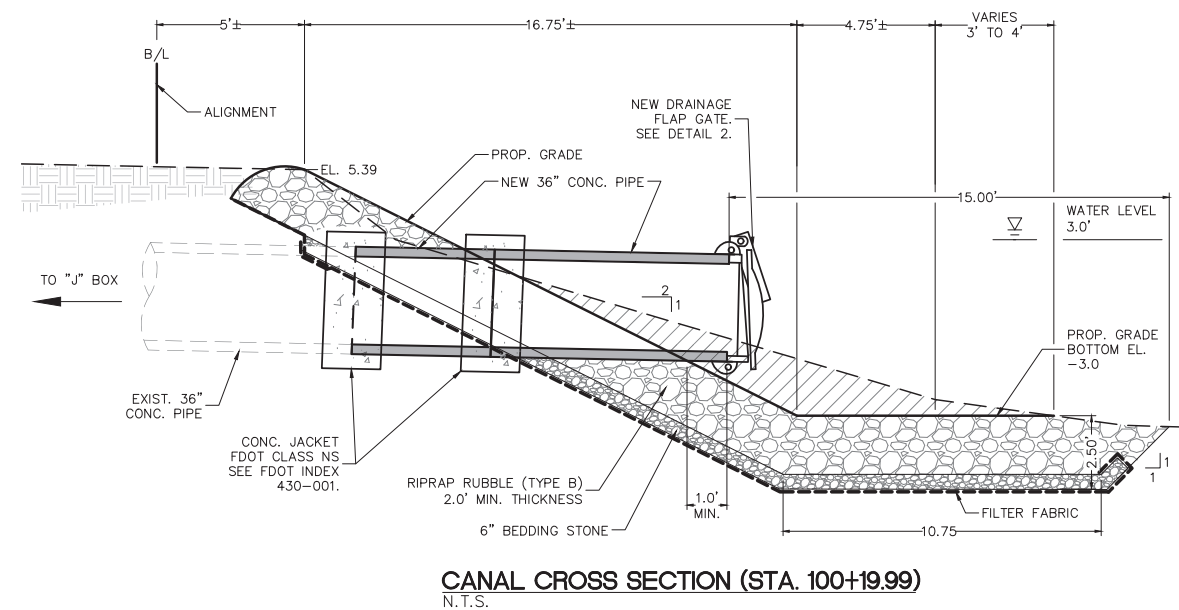
STONE FOR RIPRAP SHALL HAVE THE FOLLOWING PROPERTIES:

TEST NAME AND METHOD		TEST VALUE CRITERIA	
PROPERTY TEST	TEST METHOD	LIMESTONE	GRANITE
UNIT WEIGHT (SATURATED SURFACE-DRY)	ASTM C127	145 PCF MIN	165 PCF MIN
SPECIFIC GRAVITY (SATURATED SURFACE-DRY)	ASTM C127	2.38 MIN	2.65 MIN
ABSORPTION	ASTM C127	5% MAX	1% MAX
SODIUM SULFATE SOUNDNESS	ASTM C88, D5240	10% MAX LOSS, 5 CYCLES	5% MAX LOSS, 5 CYCLES
LA ABRASION	ASTM C535	40% MAX LOSS, 1000 REVOLUTIONS	20% MAX LOSS, 1000 REVOLUTIONS
FREEZE AND THAWING	ASTM D5312	15% MAX, 20 CYCLES	10% MAX LOSS, 20 CYCLES
WETTING AND DRYING	ASTM D5313	1% MAX, 10 CYCLES WITH NO PROGRESSIVE CRACKING	1% MAX LOSS, 30 CYCLES WITH NO PROGRESSIVE CRACKING
DURABILITY INDEX	ASTM D3744	REFER TO DURABILITY ABSORPTION RATIO	
PETROGRAPHY	ASTM C295	FRESH, INTERLOCKING CRYSTALLINE, NO CLAY MINERALS, NO SHELL FRAGMENTS, NO VOIDS, AND NO SOLUBLE MATERIALS	FRESH, INTERLOCKING CRYSTALLINE, NO CLAY MINERALS, NO VOIDS, AND NO SOLUBLE MATERIALS

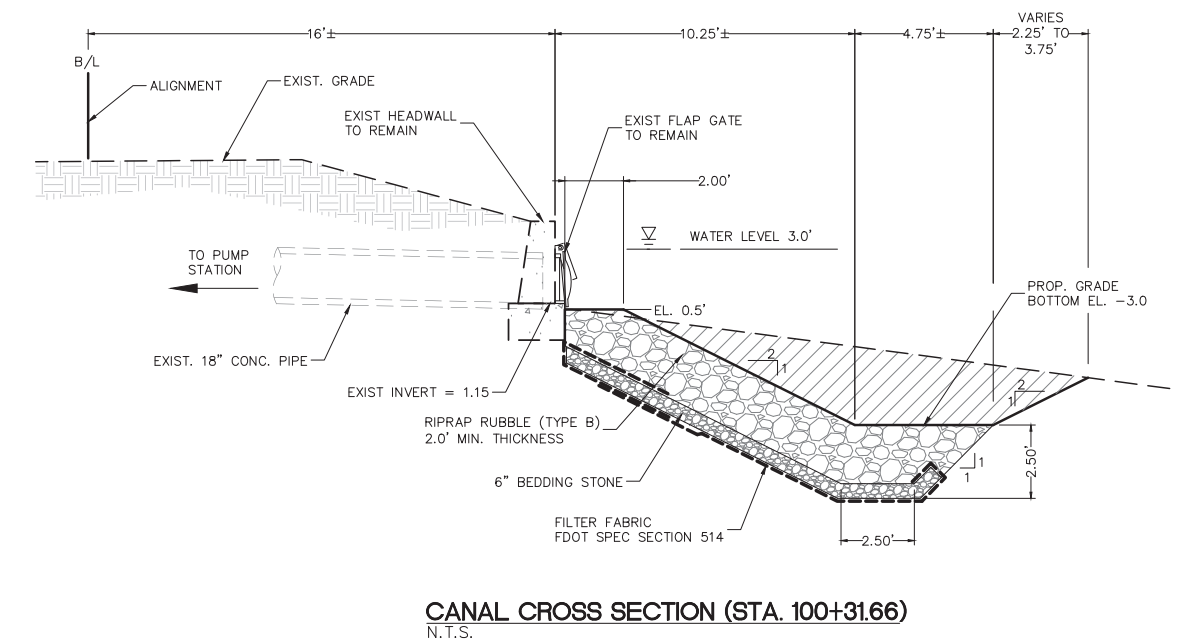
TYPE B (12-INCH AVERAGE SIZE)		
SIEVE SIZE		PERCENT PASSING BY WEIGHT
MAXIMUM	MINIMUM	
21"	15"	100
14"	12"	50
11"	8"	15

NOTES:

- ALL ELEVATIONS REFERRED TO NGVD29.
- FEMA FLOOD ZONE AE EL 7.0' NGVD29 PER FIRM PANEL 284 OF 1031, REVISED 9/11/2009.
- NEW DRAINAGE FLAP GATE TO BE WATERMAN 36 IN MODEL F-10 FOR CONCRETE PIPE, OR APPROVED EQUAL.
- RIPRAP SHALL BE TYPE B (12-IN AVERAGE SIZE), SEE TABLES FOR MORE INFORMATION.
- BEDDING STONE PER ASTM C33 SIZE NO. 1 OR 2. BEDDING STONE SHALL BE PLACED WITHIN THE LIMITS SHOWN ON THE DRAWINGS AND SHALL BE REASONABLY WELL GRADED IN ACCORDANCE WITH FDOT SECTION 901.
- MAINTAIN WATER FLOW AT ALL TIME. ADD BYPASS TO MAINTAIN FLOW IF NECESSARY DURING THE INSTALLATION OF THE PROPOSED INFLOW BOX.



CANAL CROSS SECTION (STA. 100+19.99)
N.T.S.



CANAL CROSS SECTION (STA. 100+31.66)
N.T.S.

BA

CIVIL ENGINEER / ARCHITECT:
Bermello Ajamil & Partners
4711 South LeJeune Road
Coral Gables, FL 33146
P: 305.859.2050

CIVIL / STRUCTURAL / MEP ENGINEER:
R.J.Behar & Company, Inc
Engineers - Planners
6861 SW 196th Ave suite 302,
Fort Lauderdale, FL 33332
(954) 680-7771

OWNER:
CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166

**MIAMI SPRINGS
HOOK SQUARE
PUMP STATION**
25 S. HOOK SQUARE
MIAMI SPRINGS, FL 33166

PHASE:
PERMIT SET

Note: This drawing is protected by copyright. It shall not be transmitted to any other except as agreed to by the Architect/Engineers

SEAL:

Jossmei Cruz Garcia
Date: 2024.03.18
13:03:40 -04'00'

REVISIONS:

No.:	DESCRIPTION	DATE:

TITLE:
**SITE & REVETMENT
PLAN**

Project No: 22060
Date: 10/11/2023
Scale: N.T.S.
Format: 24" x 36"
Drawn: AS
Checked: JCG

SHEET:
C-1

ATTACHMENT “C”

**REPORT OF
GEOTECHNICAL EXPLORATION**

**SOUTH HOOK SQUARE PUMP STATION
100 FEET FROM SOUTH HOOK SQUARE
MIAMI SPRINGS, FLORIDA**

FOR

**BERMELLO AJAMIL & PARTNERS, INC.
2601 SOUTH BAYSHORE DRIVE / STE. 1000
MIAMI, FLORIDA 33133**

PREPARED BY

**NUTTING ENGINEERS OF FLORIDA, INC.
2051 NW 112TH AVE, SUITE 126
MIAMI, FLORIDA 33172**

ORDER NO. 334.9

OCTOBER 2023



Geotechnical & Construction Materials
Engineering, Testing, & Inspection
Environmental Services

Offices throughout the state of Florida

www.nuttingengineers.com info@nuttingengineers.com



October 11, 2023

(Revised from September 15, 2023)

Mr. Jose Lopez
Bermello Ajamil & Partners, Inc.
2601 S. Bayshore Drive/ Ste. 1000
Miami, Florida 33133
Phone: 305-829-2050 / Cell: 954-260-5383

Email: jlopez@bermelloajamil.com

Subject: Report of Geotechnical Exploration
South Hook Square Pump Station
100 Feet from South Hook Square
Miami Springs, Florida

Dear Mr. Lopez:

Nutting Engineers of Florida, Inc. (NE), has performed a Geotechnical Exploration for the proposed wet well installation at the above referenced site in Miami Springs, Florida. This exploration was performed in accordance with the signed Subcontract for Professional Services between Bermello Ajamil & Partners, Inc. and Nutting Engineers of Florida, Inc., dated August 9, 2023. This evaluation was performed to develop limited information regarding subsurface soil conditions at specific test locations which along with proposed construction information provided were used to develop opinions regarding general earthwork procedures and foundations for support of the proposed construction. This report presents our findings and recommendations based upon the information examined at the time of this evaluation.

PROJECT INFORMATION

Per our conversation on July 24, 2023 and review of the aerial provided, and the outfall refurbishment plan provided on September 7, 2023, we understand that plans for this project include replacing the existing inflow box structure located to the west of the existing pump station and refurbishing the outfall of the existing canal at the referenced site. We understand that the bottom depth of the inflow box approximately seven feet below existing grades. The existing pump station will remain and no new wet wells will be installed. We understand that plans for outfall refurbishment include excavation/dredging and restoring the existing canal outfall with rip rap. The design of the outfall refurbishment is being performed by others.

GENERAL SUBSURFACE CONDITIONS

Soil Survey Maps

As part of the geotechnical study, we reviewed the Department of Agriculture Soil Survey of Miami-Dade County. These SCS maps provide qualitative information about potential general shallow soil conditions in the project vicinity. This information was derived from approximately 6 ft. deep manual auger borings, aerial photo and surface feature interpretation at some point in the past (mid 1980's to early 1970's). The SCS data may or may not reflect actual current site conditions. A review of the Soil Survey for Dade County revealed that at the time the survey was conducted, the soils at the site were described as Urban land. This map unit is in areas where shopping centers, parking lots, streets, sidewalks, airports, large buildings, houses, and other structures cover more than 85 percent of the surface. The natural soil cannot be observed. The soils in open areas, mostly lawns, vacant lots, playgrounds, and parks are mainly Udorthents. We note that the maximum depth of the survey is approximately 6 feet.

Subsurface Exploration

NUTTING ENGINEERS OF FLORIDA, INC. performed one (1) Standard Penetration Test (SPT) borings (ASTM D-1586) to a depth of thirty feet below land surface. The location of the test boring is indicated on the boring location plan presented in the Appendix of this report. The boring location was identified in the field using approximate methods; namely, a measuring wheel and available surface controls. As such the test boring location should be considered to be approximate.

Test Boring Results

In general, the test boring recorded a surficial layer of approximately ten inches of topsoil underlain by medium dense to very loose light gray to light brown fine sand to depth of about five feet. Soft light brown and light gray limestone with fine sand was then encountered to a depth of about seventeen feet. The limestone formation was underlain by loose light gray fine sand (with or without limestone fragments) to thirty feet, the maximum depth explored. A detailed description of the soil/rock interlayering is given on the test boring log in the Appendix.

Rock Formation Note: Generally, rock in the South Florida area may include limestone or sandstone which have irregularities and discontinuities including vertical and horizontal solution features, varying surface and bottom elevations, and varying degrees of hardness. The rock features may also contain intervening sand/silt and other material filled lenses. When these features are penetrated with a split spoon, very loose soil is typically indicated even though the rock surrounding the soil filled solution hole may be very hard. Both horizontal and vertical discontinuities in the rock strata can complicate excavation and dewatering operations substantially. The standard penetration test boring executed in this evaluation was performed in accordance with the normal standard of care in this area. Despite this, this process may sometimes fail to detect the presence of rock strata by passing through solution features.

Solution features can be very common in rock strata in Southeast Florida. For these reasons it is strongly recommended that underground work, dewatering design and implementation be performed by parties with extensive local experience with similar types of installation.

Groundwater Information

The immediate groundwater level was measured at the boring location at the time of drilling. The groundwater level was encountered at approximately five feet below the existing ground surface at the time of drilling.

The immediate depth to groundwater measurements presented in this report may not provide a reliable indication of stabilized or longer-term depth to groundwater at this site. Water table elevations can vary dramatically with time through rainfall, droughts, storm events, flood control activities, nearby surface water bodies, tidal activity, pumping and many other factors. For these reasons, this immediate depth to water data **should not** be relied upon alone for project design considerations.

ANALYSIS AND RECOMMENDATIONS

Proposed Inflow Box Structure

The borings performed for this project suggests that the soils encountered are suitable for support of the proposed inflow box structure using a shallow foundation system, based on an anticipated bottom depth of seven feet below existing grades. Once the site is successfully prepared in accordance with the recommendations presented in this report, the site may be developed with the proposed new inflow box structure using a shallow foundation system designed for an allowable soil bearing pressure of 3,000 pounds per square foot.

Once plans are more finalized for the proposed construction, we should review the plans to determine whether additional details or changes to our recommendations are warranted. All work should be completed in accordance with applicable codes, other regulations as appropriate, and good standard local practice.

Site Preparation – Proposed Inflow Box Structure

The surficial organic soils, debris from the clearing operations, and any unsuitable soils as determined by the Geotechnical Engineer will need to be completely removed within the construction area and to a lateral distance of at least three feet beyond the footprint limits. A Nutting Engineer's representative should be present to observe that the stripping operations are performed as we have discussed herein.

When this has been completed, the proposed replacement inflow box structure area should be excavated to the required depth which we understand will be approximately seven feet. We were not provided specific plans indicating whether the box will be pre-cast or cast-in-place. Therefore, if dewatering is needed, the dewatering plan and implementation should be performed by knowledgeable individuals having local experience.

Excavations

An unsupported vertical cut is not considered stable or safe during construction. An unsupported excavation may cause the collapse of the sidewalls. Therefore, in our opinion and following the current regulations established by OSHA for excavations, cut slopes may be required depending on the method of installation. It is our opinion that if the excavation remains dry, temporary side slopes of 3 horizontal to 1 vertical may be used for this project. If slopes cannot be maintained, then the excavation must be cased or shored. Shoring procedures should conform to those presented in the Occupational Safety and Health Administration (OSHA) standards.

Subsurface Soil Engineering Properties – Proposed Outfall Refurbishment

In order to provide design parameters for the proposed outfall refurbishment, the values given in the accompanying table should be consulted. The table is based on visual classification, empirical relationships and our experience with similar soil conditions. If more exact values are needed, we recommend that specific laboratory tests be performed.

The borings performed for this project generally encountered sands to depth of about five feet, underlain by a limestone formation extending to a depth of about seventeen feet, followed by sands with limestone fragments or shells to depth of thirty feet. Groundwater was encountered at a depth of about five feet, with an estimated elevation of +3 feet NAVD, based on an estimated ground surface elevation of +7 to +8 feet NAVD. The canal stabilization plans indicate that the maximum water depth in the area of the improvements is approximately six feet (-3 feet NAVD), with the canal water surface at approximately +3 feet NAVD. We understand that rip rap will be used to provide a slope of 2 horizontal to 1 vertical. If steeper slopes are needed, we should be notified to provide alternatives and discussions should be held with all interested parties to determine the best alternative.

GENERAL INFORMATION

Our client for this geotechnical evaluation was:

Bermello Ajamil & Partners, Inc.
2601 S. Bayshore Drive/ Ste. 1000
Miami, Florida 33133

The contents of this report are for the exclusive use of the client, the client's design & construction team and governmental authorities for this specific project exclusively. Information conveyed in this report shall not be used or relied upon by other parties or for other projects without the expressed written consent of Nutting Engineers of Florida, Inc. This report discusses geotechnical considerations for this site based upon observed conditions and our understanding of proposed construction for foundation support. Environmental issues including (but not limited to), soil and/or groundwater contamination, and other environmental considerations are beyond our scope of service for this project. As such, this report should not be used or relied upon for evaluation of environmental issues.

If conditions are encountered which are not consistent with the findings presented in this report, or if proposed construction is moved from the location investigated, this office shall be notified immediately so that the condition or change can be evaluated, and appropriate action taken.

Nutting Engineers of Florida, Inc. (NE), recommends that we be contracted to provide input to the design team and owner during the foundation and earthwork design process and that we review final foundation drawings and specifications to verify that our report recommendations and design intent have been properly implemented. NE shall also perform testing during the earthwork and foundation construction as recommended in this report. If NE is not engaged to perform these services as detailed herein, the Client agrees that NE shall bear no liability for the interpretation, implementation of our report, its recommendations and/or inspection and testing services as described in this report if implemented by others.

The Geotechnical Engineer warrants that the findings, recommendations, specifications, or professional advice contained herein, have been presented after being prepared in accordance with general accepted professional practice in the field of foundation engineering, soil mechanics and engineering geology. No other warranties are implied or expressed.

We appreciate the opportunity to be of service on this project. If we can be of any further assistance, or if you need additional information, please contact us at your convenience.

Sincerely,
NUTTING ENGINEERS OF FLORIDA, INC.

Colin T. Henderson, P.E. #96283
Project Engineer

Richard Wohlfarth, P.E #50858
Principal / Director of Engineering

Appendix: Boring Location Plan
 Test Boring Result
 Soil Properties Table
 Limitations of Liability
 Soil Classification Criteria



- LEGEND -
 TEST BORING LOCATION



Bermello Ajamil & Partners
Miami Springs Hook Square
61 South Hook Square
Miami Springs, Florida 33166
PROJECT NO. 334.9

APPROXIMATE
TEST LOCATION
PLAN

GEOTECHNICAL EXPLORATION

FIG. 1



PROJECT NUMBER 334.9

CLIENT Bermello Ajamil & Partners

PROJECT NAME Miami Springs Hook Square

PROJECT LOCATION 100' from South Hook Square, Miami Springs, FL

DATE STARTED 8/30/23 COMPLETED 8/30/23 SURFACE ELEVATION REFERENCE Same as road crown

DRILLING METHOD Standard Penetration Boring GROUND WATER LEVELS:

LOGGED BY Dancor CHECKED BY C. Henderson ∇ AT TIME OF DRILLING 4.8 ft

APPROXIMATE LOCATION OF BORING As located on site plan

TEST NUTTING BOREHOLE 2-334.9 BERMELO AJAMIL & PARTNERS - MIAMI SPRINGS HOOK SQUARE.GPJ GINT US.GDT 9/6/23

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	Blows	N-Value	▲ SPT N VALUE ▲			
						10	20	30	40
						PL	MC	LL	
						20	40	60	80
						□ FINES CONTENT (%) □			
						20	40	60	80
0		TOPSOIL, trace limerock fragments (10" thick)							
		Lt. gray fine SAND	SS 1	6-5-5-5	10	▲			
		Lt. brown fine SAND	SS 2	3-2-WOH-WOH					
5	∇	Lt. brown LIMESTONE with fine SAND	SS 3	1-2-8-12	10	▲			
			SS 4	12-16-9-5	25			▲	
			SS 5	5-4-5-5	9	▲			
		Lt. brown and lt. gray LIMESTONE with fine SAND	SS 6	5-3-6-5	9	▲			
		Lt. gray fine SAND with LIMESTONE	SS 7	3-3-3-3	6	▲			
		Lt. gray fine SAND, trace limestone fragments	SS 8	4-4-5-4	9	▲			
		Lt. gray fine SAND, trace shells	SS 9	3-3-6-6	9	▲			
30		Bottom of hole at 30.0 feet.							

**TABLE OF SOIL PROPERTIES
BORING B-1 (9/8/2023)**

Depth (feet)	Description	Unit Weight (lb./cu.ft)		Angle of Internal Friction (Degrees)	Earth Pressure Coefficient		
		Saturated	Submerged		Passive	Active	At Rest
0' – 3'	Medium dense Fine SAND	115	52.6	30	3.0	0.33	0.5
3' – 5'	Very loose Fine Sand	105	42.6	26	2.56	0.39	0.56
5' – 17'	LIMESTONE with fine SAND	135	72.6	0	1.0	1.0	1.0
17' – 22'	Loose fine SAND with limestone fragments	115	52.6	28	2.77	0.36	0.53
22' – 30'	Loose fine SAND, trace limestone or shell fragments	115	52.6	29	2.88	0.35	0.52

Appropriate Factors of Safety should be used in the foundation design.

Note: Groundwater (WT) was encountered at a depth of about 5 feet below existing ground surface at time drilling performed

LIMITATIONS OF LIABILITY

WARRANTY

We warrant that the services performed by Nutting Engineers of Florida, Inc. are conducted in a manner consistent with that level of care and skill ordinarily exercised by members of the profession in our area currently practicing under similar conditions at the time our services were performed. **No other warranties, expressed or implied, are made.** While the services of Nutting Engineers of Florida, Inc. are a valuable and integral part of the design and construction teams, we do not warrant, guarantee or insure the quality, completeness, or satisfactory performance of designs, construction plans, specifications we have not prepared, nor the ultimate performance of building site materials or assembly/construction.

SUBSURFACE EXPLORATION

Subsurface exploration is normally accomplished by test borings; test pits are sometimes employed. The method of determining the boring location and the surface elevation at the boring is noted in the report. This information is represented in the soil boring logs and/or a drawing. The location and elevation of the borings should be considered accurate only to the degree inherent with the method used and may be approximate.

The soil boring log includes sampling information, description of the materials recovered, approximate depths of boundaries between soil and rock strata as encountered and immediate depth to water data. The log represents conditions recorded specifically at the location where and when the boring was made. Site conditions may vary through time as will subsurface conditions. The boundaries between different soil strata as encountered are indicated at specific depths; however, these depths are in fact approximate and dependent upon the frequency of sampling, nature and consistency of the respective strata. Substantial variation between soil borings may commonly exist in subsurface conditions. Water level readings are made at the time and under conditions stated on the boring logs. Water levels change with time, precipitation, canal level, local well drawdown and other factors. Water level data provided on soil boring logs shall not be relied upon for groundwater based design or construction considerations.

LABORATORY AND FIELD TESTS

Tests are performed in *general* accordance with specific ASTM Standards unless otherwise indicated. All criteria included in a given ASTM Standard are not always required and performed. Each test boring report indicates the measurements and data developed at each specific test location.

ANALYSIS AND RECOMMENDATIONS

The geotechnical report is prepared primarily to aid in the design of site work and structural foundations. Although the information in the report is expected to be sufficient for these purposes, it shall not be utilized to determine the cost of construction nor to stand alone as a construction specification. Contractors shall verify subsurface conditions as may be appropriate prior to undertaking subsurface work.

Report recommendations are based primarily on data from test borings made at the locations shown on the test boring reports. Soil variations commonly exist between boring locations. Such variations may not become evident until construction. Test pits sometimes provide valuable supplemental information that derived from soil borings. If variations are then noted, the geotechnical engineer shall be contacted in writing immediately so that field conditions can be examined and recommendations revised if necessary.

The geotechnical report states our understanding as to the location, dimensions and structural features proposed for the site. **Any significant changes of the site improvements or site conditions must be communicated in writing to the geotechnical engineer immediately** so that the geotechnical analysis, conclusions, and recommendations can be reviewed and appropriately adjusted as necessary.

CONSTRUCTION OBSERVATION

Construction observation and testing is an important element of geotechnical services. The geotechnical engineer's field representative (G.E.F.R.) is the "owner's representative" observing the work of the contractor, performing tests and reporting data from such tests and observations. **The geotechnical engineer's field representative does not direct the contractor's construction means, methods, operations or personnel.** The G.E.F.R. does not interfere with the relationship between the owner and the contractor and, except as an observer, does not become a substitute owner on site. The G.E.F.R. is responsible for his/her safety, but has no responsibility for the safety of other personnel at the site. The G.E.F.R. is an important member of a team whose responsibility is to observe and test the work being done and report to the owner whether that work is being carried out in general conformance with the plans and specifications. The enclosed report may be relied upon solely by the named client.

SOIL AND ROCK CLASSIFICATION CRITERIA

SAND/SILT

N-VALUE (bpf)	RELATIVE DENSITY
0 – 4	Very Loose
5 – 10	Loose
11 – 29	Medium
30 – 49	Dense
>50	Very dense
100	Refusal

CLAY/SILTY CLAY

N-VALUE (bpf)	UNCONFINED COMP. STRENGTH (tsf)	CONSISTENCY
<2	<0.25	v. Soft
2 – 4	0.25 – 0.50	Soft
5 – 8	0.50 – 1.00	Medium
9 – 15	1.00 – 2.00	Stiff
16 – 30	2.00 – 4.00	v. Stiff
>30	>4.00	Hard

ROCK

N-VALUE (bpf)	RELATIVE HARDNESS	ROCK CHARACTERISTICS
$N \geq 100$	Hard to v. hard	Local rock formations vary in hardness from soft to very hard within short vertical and horizontal distances and often contain vertical solution holes of 3 to 36 inch diameter to varying depths and horizontal solution features. Rock may be brittle to split spoon impact, but more resistant to excavation.
$25 \leq N \leq 100$	Medium hard to hard	
$5 \leq N \leq 25$	Soft to medium hard	

PARTICLE SIZE

Boulder	>12 in.
Cobble	3 to 12 in.
Gravel	4.76 mm to 3 in.
Sand	0.074 mm to 4.76 mm
Silt	0.005 mm to 0.074 mm
Clay	<0.005 mm

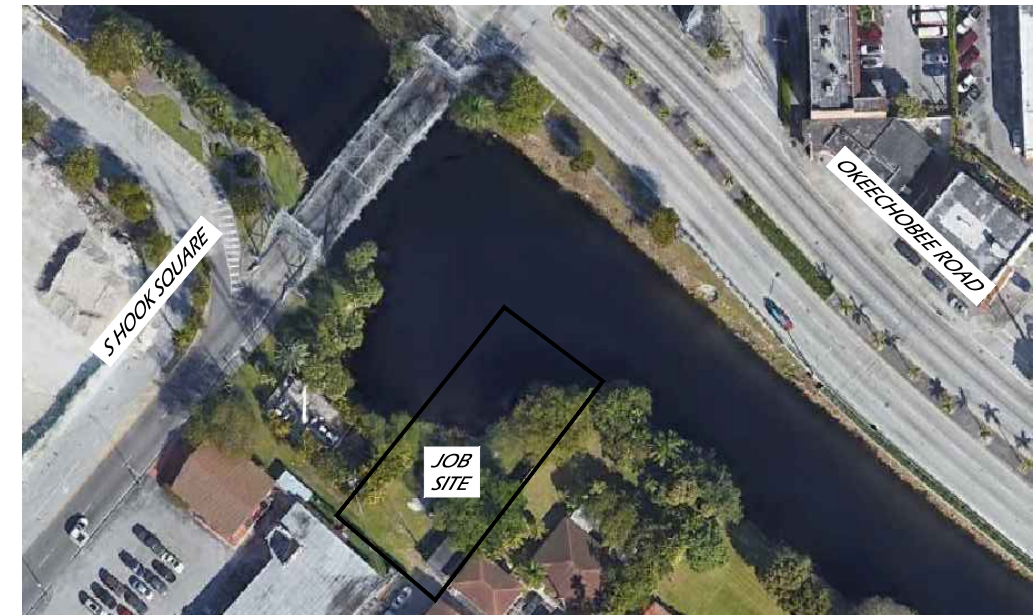
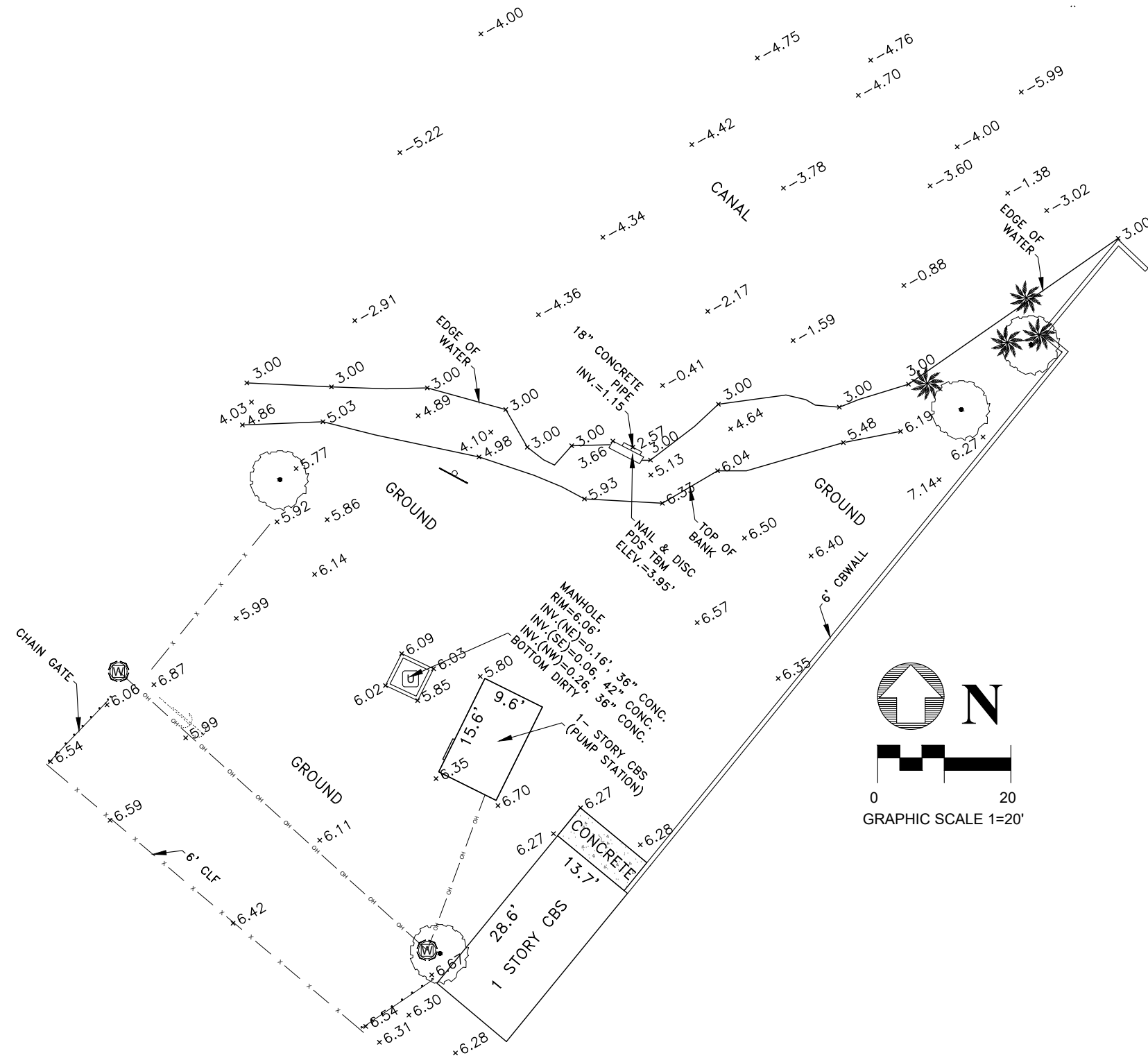
DESCRIPTION MODIFIERS

0 – 5%	Slight trace
6 – 10%	Trace
11 – 20%	Little
21 – 35%	Some
>35%	And

Major Divisions		Group Symbols	Typical names	Laboratory classification criteria		
Coarse-grained soils (More than half of material is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	Clean gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows: Less than five percent.....GW, GP, SW, SP More than 12 percent.....GM, GC, SM, SC 5 to 12 percent.....Borderline cases requiring dual systems**	
		Poorly graded gravels, gravel-sand mixtures, little or no fines	GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		
		Gravels with fines (Appreciable amount of fines)	GW*	d		Silty gravels, gravel-sand-silt mixtures
			u			
	GC	Clayey gravels, gravel-sand-clay mixtures				
	Sands (More than half of coarse fraction is smaller than No. 4 sieve size)	Clean sands (Little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines		
		Poorly graded sands, gravelly sands, little or no fines	SP	Poorly graded sands, gravelly sands, little or no fines		
		Sands with fines (Appreciable amount of fines)	SM*	d		Silty sands, sand-silt mixtures
			u			
		SC	Clayey sands, sand-clay mixtures			
		$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_z = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 Not meeting all gradation requirements for GW Atterberg limits below "A" line or P.I. less than 4 Atterberg limits above "A" line with P.I. greater than 7 $C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_z = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 Not meeting all gradation requirements for SW Atterberg limits below "A" line or P.I. less than 4 Atterberg limits above "A" line with P.I. more than 7 Limits plotting in hatched zone with P.I. between 4 and 7 are borderline cases requiring use of dual system.				
Fine-grained soils (More than half of material is smaller than No. 200 sieve size)	Silt and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity			
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy, clays, silty clays, lean clays			
		OL	Organic silts and organic silty clays of low plasticity			
	Silt and clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts			
		CH	Inorganic clays or high plasticity, fat clays			
		OH	Organic clays of medium to high plasticity, organic silts			
	Highly organic soils	PT	Peat and other highly organic soils			
					<p style="text-align: center;">Plasticity Chart</p>	

ATTACHMENT “D”

SPECIFIC PURPOSE SURVEY



LOCATION SKETCH
NOT TO SCALE

PROPERTY ADDRESS
25 S HOOK SQ, CITY OF MIAMI SPRINGS, FLORIDA 33166

PREPARED FOR:
CITY OF MIAMI SPRINGS, FLORIDA

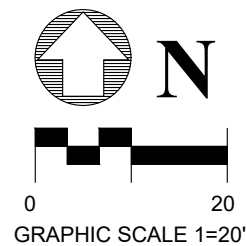
SURVEYOR'S NOTES:

1. NO UNDERGROUND PORTIONS OF FOOTINGS, FOUNDATIONS. CONTACT APPROPRIATE AUTHORITY PRIOR TO ANY DESIGN WORK OR CONSTRUCTION.
2. THIS IS NOT A BOUNDARY SURVEY. EXAMINATION OF ABSTRACT OF TITLE MUST BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING PROPERTY BESIDES THOSE NOTED.
3. NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.
4. TYPE OF SURVEY: SPECIFIC PURPOSE.
5. ELEVATIONS ARE REFERRED TO THE NATIONAL GEODETIC VERTICAL DATUM 1929. USED BENCHMARK: M-75-1. ELEVATION: 7.61 (NORTH GEODETIC VERTICAL DATUM 1929)
6. FIELD WORK DATE: 01.20.2023

SYMBOLS & ABBREVIATIONS

CBS = CONCRETE BLOCK STRUCTURE; CBWALL= CONCRETE BLOCK WALL; CL = CENTER LINE; CLF = CHAIN LINK FENCE; EOP = EDGE OF PAVEMENT; FND= FOUND; FFE = FINISH FLOOR ELEVATION; I.D.= IDENTIFICATION; INV.= INVERT ELEVATION; IP = IRON PIPE; MDCR = MIAMI DADE COUNTY RECORDS; NAC = NON ACCESSIBLE CORNER; PB = PLAT BOOK; PG = PAGE; RB = REBAR; RDS= PREMIER DESIGN SOLUTIONS; UE = UTILITY EASEMENT;

NOTE: SYMBOLS SHOWN ARE NOT TO SCALE AND SHALL NOT BE USED TO SIZE SUCH ELEMENTS.



PREMIERE DESIGN SOLUTIONS INC.
11606 CITY HALL PROMENADE • STE 200
• MIRAMAR FL, 33025
954.237.7850
PDS@PDS-ENG.COM
FLORIDA COA No. 27940
LB 8017

REVISIONS			
No.	DATE	REMARKS	BY

CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166



SPECIFIC PURPOSE SURVEY

25 S HOOK SQ
MIAMI SPRINGS, FL 33166

FERNANDO FERNANDEZ
PROFESSIONAL SURVEYOR AND MAPPER
LICENSE LS-6765 STATE OF FLORIDA
ffernandez@pds-eng.com

DATE: 02.14.2023

ATTACHMENT “E”

ATM Engineering

TESTING LABORATORIES-ENGINEERING-INSPECTION SERVICES -DRILLING ENVIRONMENTAL SERVICES.

AUBREY ENGINEERING, LLC d/b/a ATM Engineering

1950 West 84th Street, Hialeah, Florida 33014/Phone: 305-646-1888/Fax: 305-646-1887

11 March 2024

JOSE L. LOPEZ, PE, PMP, ENV SP
Director of Environmental Engineering
Bermello Ajamil & Partners
4711 S Le Jeune Rd
Coral Gables, FL 33146

RE : Miami Springs pump station
Concrete Testing

Dear Jose,

As directed, ATM Engineering tested the concrete of the pump station located at

25 S Hook Square, Miami Gardens

We took two (2) samples from locations as you directed, and one (1) sample from the outfall itself. The third sample was collected because it appeared to be a different concrete than the first two locations. Sample locations are shown in the drawing attached to this report, as well as pictures of the locations during sample collection.

Samples were placed in individual sealed containers to avoid any cross-contamination.

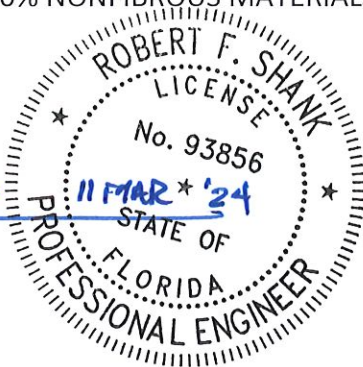
We tested the concrete samples for the presence of Asbestos materials as requested.

ALL SAMPLES CONTAINED 100% NONFIBROUS MATERIALS, indicating that the presence of Asbestos materials is NEGATIVE.

Regards,



Robert Shank, PE #93856



Encl: Sample location Photos
Site Map

ATM Engineering

TESTING LABORATORIES-ENGINEERING-INSPECTION SERVICES -DRILLING ENVIRONMENTAL SERVICES.

AUBREY ENGINEERING, LLC d/b/a ATM Engineering

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PHOTOS

SAMPLE LOCATION 1 – REINFORCED CONCRETE PIPE



ATM Engineering

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SAMPLE LOCATION 2 – CORNER OF MANHOLE/JUNCTION BOX



ATM Engineering

TESTING LABORATORIES-ENGINEERING-INSPECTION SERVICES -DRILLING ENVIRONMENTAL SERVICES.

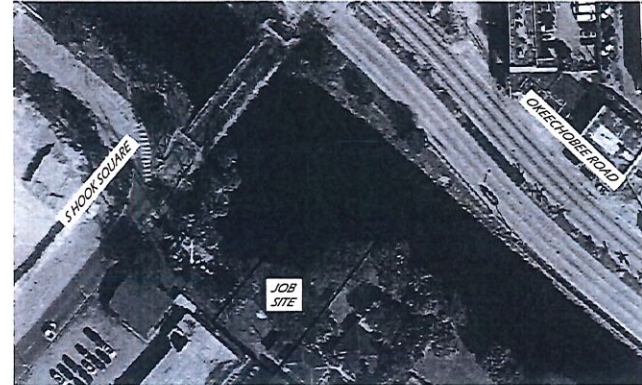
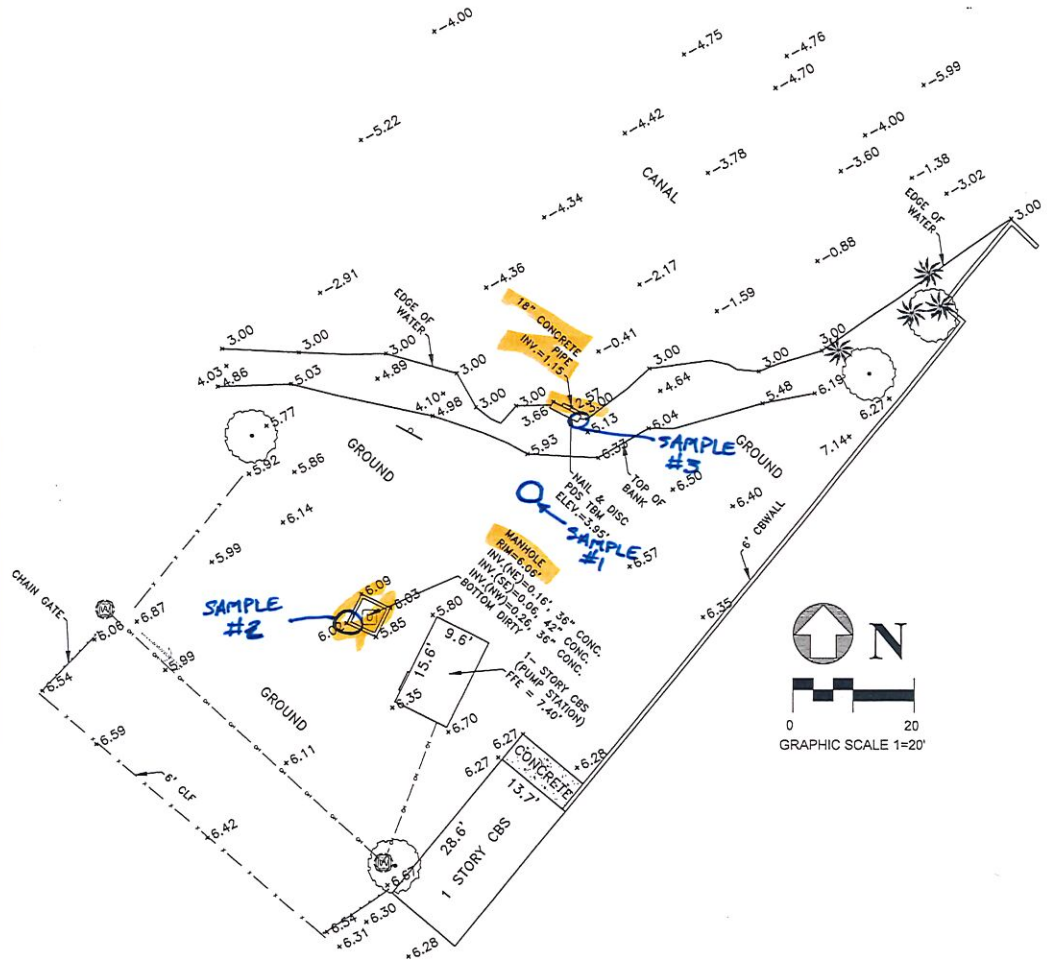
AUBREY ENGINEERING, LLC d/b/a ATM Engineering

1950 West 84th Street, Hialeah, Florida 33014/Phone: 305-646-1888/Fax: 305-646-1887

SAMPLE LOCATION 3 – TOP EDGE OF OUTFALL



SPECIFIC PURPOSE SURVEY



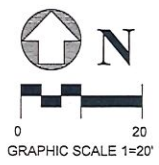
LOCATION SKETCH
NOT TO SCALE

PROPERTY ADDRESS
25 S HOOK SQ, CITY OF MIAMI SPRINGS, FLORIDA 33166

PREPARED FOR:
CITY OF MIAMI SPRINGS, FLORIDA

- SURVEYOR'S NOTES:**
1. NO UNDERGROUND PORTIONS OF FOOTINGS, FOUNDATIONS. CONTACT APPROPRIATE AUTHORITY PRIOR TO ANY DESIGN WORK OR CONSTRUCTION.
 2. THIS IS NOT A BOUNDARY SURVEY. EXAMINATION OF ABSTRACT OF TITLE MUST BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING PROPERTY BESIDES THOSE NOTED.
 3. NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.
 4. TYPE OF SURVEY: SPECIFIC PURPOSE.
 7. ELEVATIONS ARE REFERRED TO THE NATIONAL GEODETIC VERTICAL DATUM 1929. USED BENCHMARK: M-75-1. ELEVATION: 7.61 (NORTH GEODETIC VERTICAL DATUM 1929)
 8. FIELD WORK DATE: 01.20.2023

SYMBOLS & ABBREVIATIONS
 CBS = CONCRETE BLOCK STRUCTURE; CBWALL= CONCRETE BLOCK WALL; CL = CENTER LINE; CLF = CHAIN LINK FENCE; EOP = EDGE OF PAVEMENT; FND= FOUND; FFE = FINISH FLOOR ELEVATION; I.D.= IDENTIFICATION; INV.= INVERT ELEVATION; IP = IRON PIPE; MDCR = MIAMI DADE COUNTY RECORDS; NAC = NON ACCESSIBLE CORNER; PB = PLAT BOOK; PG = PAGE; RB = REBAR; RDS= PREMIER DESIGN SOLUTIONS; UE = UTILITY EASEMENT;



NOTE: SYMBOLS SHOWN ARE NOT TO SCALE AND SHALL NOT BE USED TO SIZE SUCH ELEMENTS.

PREMIERE DESIGN SOLUTIONS INC.
11606 CITY HALL PROMENADE • STE 200
• MIRAMAR FL 33025
954.237.7652
PDS@PDS-ENG.COM
FLORIDA COA No. 27940
LD 8017

REVISIONS			
No.	DATE	REMARKS	BY

CITY OF MIAMI SPRINGS
201 WESTWARD DR
MIAMI SPRINGS, FL 33166

SPECIFIC PURPOSE SURVEY

25 S HOOK SQ
MIAMI SPRINGS, FL 33166

FERNANDO FERNANDEZ
PROFESSIONAL SURVEYOR AND MAPPER
LICENSE LS-6766 STATE OF FLORIDA
fernandez@pds-eng.com

DATE: 02.14.2023

ATTACHMENT “F”

CITY OF MIAMI SPRINGS

Construction of The Hook Square Pump House Replacement Project

City of Miami Springs Project No.: ITB No.: 04-23/24

Engineering Company: Bermello Ajamil and Partners

Contractor Name:

Contractor Address:

Date of submission:



Pay Item Number	Description of Work	Quantity	Unit	Bidder's Unit Cost	Total Cost
DIV .2	Demolition / Removal of Miscellaneous Items and Connections, Additional Minor Repair and Preparation, removal of existing generator, mechanical and structural	1	LS		
02 81 20	Removal of Hazardous Waste, Remediation	1	LS		
DIV .3	CONCRETE, new concrete slab, roof and concrete beam	1	LS		
DIV .4	MASONRY- Stucco and paint	1	LS		
DIV .5	METALS- Structural framing	1	LS		
DIV .6	WOODS AND PLASTICS- Fiberglass grating	1	LS		
DIV .7	THERMAL AND MOISTURE PROTECTION- Roofing specialties, fireproofing and roof hatch, joint sealants, caulking and waterproofing	1	LS		
DIV .8	OPENINGS- Steel doors and frames, Door Hardware, Weatherstripping, Thresholds and Seals	1	LS		
DIV .22	PLUMBING	1	LS		
22.11.1	Installation/Replacement to Existing Fiarbanks Pump PM7068. Installation of New National Pump Model #20-LL20P Propeller	1	LS		

CITY OF MIAMI SPRINGS

Construction of The Hook Square Pump House Replacement Project
 City of Miami Springs Project No.: ITB No.: 04-23/24
 Engineering Company: Bermello Ajamil and Partners



Contractor Name:
 Contractor Address:

Date of submission:

Pay Item Number	Description of Work	Quantity	Unit	Bidder's Unit Cost	Total Cost
DIV .26	ELECTRICAL- Lighting control devices, low voltage distribution equipment and wiring devices, interior lightning, conduct & wire, raceways	1	LS		
DIV .31	EARTHWORK- Site clearing, earth moving, excavation, fill, erosion and sediment controls, riprap	1	LS		
DIV .33	UTILITIES- Storm drainage, utility precast box, drainage grate	1	LS		
	Miscellaneous Connections & Adjustments (Bends, Valves, Plugs, etc..)	1	LS		
	PUMP CHARGES - PUMP HAS BEEN PURCHASED BY THE CITY				
	OPTIONAL EQUIPMENT				
	Emergency generator- Portable				
	Bid bond		5%		
	Performance Bond		%		
	Payment Bond		%		
	TOTAL BID AMOUNT- LUMP SUM				
	Additional lines, as needed				

ATTACHMENT “G”

**AMENDMENT NO. 1
TO AGREEMENT NO. LPA0336
BETWEEN
FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION
AND
CITY OF MIAMI SPRINGS**

This Amendment to Agreement No. LPA0336 (Agreement) is made by and between the Department of Environmental Protection (Department), an agency of the State of Florida, and City of Miami Springs (Grantee), on the date last signed below.

WHEREAS, the Department entered into the Agreement with the Grantee for Miami Springs Hook Square Pump House Replacement (Project), effective July 1, 2022; and,

WHEREAS, the Grantee has requested an extension of the Agreement due to supply chain issues delaying project implementation; and,

WHEREAS, other changes to the Agreement are necessary.

NOW THEREFORE, the parties agree as follows:

1. Section 3. of the Standard Grant Agreement is hereby revised to change the Date of Expiration to April 30, 2025. The Department and the Grantee shall continue to perform their respective duties during this extension period pursuant to the same terms and conditions provided in the Agreement.
2. Section 4. of Attachment 2 is hereby revised to the following:

Reimbursement for costs or availability for costs to meet matching requirements shall be limited to the following budget categories, as defined in the Reference Guide for State Expenditures, as indicated:

<u>Reimbursement</u>	<u>Match</u>	<u>Category</u>
<input type="checkbox"/>	<input type="checkbox"/>	Salaries/Wages
		Overhead/Indirect/General and Administrative Costs:
<input type="checkbox"/>	<input type="checkbox"/>	a. Fringe Benefits, for actual costs not to exceed the budget amount identified in Attachment 3.
<input type="checkbox"/>	<input type="checkbox"/>	b. Indirect Costs, for actual costs not to exceed the budget amount identified in Attachment 3.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Contractual (Subcontractors)
<input type="checkbox"/>	<input type="checkbox"/>	Travel, in accordance with Section 112, F.S.
<input type="checkbox"/>	<input type="checkbox"/>	Equipment
<input type="checkbox"/>	<input type="checkbox"/>	Rental/Lease of Equipment
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Miscellaneous/Other Expenses
<input type="checkbox"/>	<input type="checkbox"/>	Land Acquisition

3. Attachment 3, Grant Work Plan, is hereby deleted in its entirety and replaced with Attachment 3-1, Revised Grant Work Plan, as attached to this Amendment and hereby incorporated into the Agreement. All references in the Agreement to Attachment 3 shall hereinafter refer to Attachment 3-1, Revised Grant Work Plan.

4. All other terms and conditions of the Agreement remain in effect. If and to the extent that any inconsistency may appear between the Agreement and this Amendment, the provisions of this Amendment shall control.

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**ATTACHMENT 3-1
REVISED GRANT WORK PLAN**

PROJECT TITLE: Hook Square Pump House Replacement

PROJECT LOCATION: The Project will be located in the City of Miami Springs within Miami-Dade County; Lat/Long (25.8202, -80.2808). See Figure 1 for location map.

PROJECT BACKGROUND: The City of Miami Springs (Grantee) has determined that the Hook Square pump station has deteriorated to the point that it is no longer meeting the needs of the community. The pump station and related appurtenances needs to be replaced to protect water quality, minimize flood risks, provide bank stabilization, and protect local manatee populations. The Grantee will purchase the following equipment that will be permanently installed by the subcontractor: National Pump Model Propeller Pump for 10,000 GPM.

PROJECT DESCRIPTION: The Grantee will replace a 35 HP pump station along the C-6 canal. The replacement includes the installation of new 50 HP axial flow pump, portable generator, electric controls, refurbishing of bar screens as needed, pump discharge and bypass piping, as well as restoration of existing buildings and headwalls including an outflow with flap-gate and rip rap bank stabilization.

TASKS: All documentation should be submitted electronically unless otherwise indicated.

Task 1: Preconstruction Activities

Deliverables: The Grantee will complete a topographical and bathymetric survey, geotechnical report and existing conditions report to produce a pre-design report. The Grantee will complete the design and calculations for the replacement of the existing pump, structural, mechanical and electrical plans, and bank stabilization and obtain all necessary permits for construction of the project.

Documentation: The Grantee will submit: 1) a signed acceptance of the completed work to date, as provided in the Grantee's Certification of Payment Request; and 2) a summary of design (or preconstruction) activities to date, indicating the percentage of design completion for the time period covered in the payment request. For the final documentation, the Grantee will also submit a copy of the design completed with the funding provided for this task and a list of all required permits identifying issue dates and issuing authorities.

Performance Standard: The Department's Grant Manager will review the documentation to verify that the deliverables have been completed as described above. Upon review and written acceptance by the Department's Grant Manager, the Grantee may proceed with payment request submittal.

Payment Request Schedule: The Grantee may submit a payment request for cost reimbursement no more frequently than monthly.

Task 2: Bidding and Contractor Selection

Deliverables: The Grantee will prepare a bid package, publish a public notice, solicit bids, conduct pre-bid meetings, and respond to bid questions in accordance with the Grantee's procurement process, to select one or more qualified and licensed contractors to complete construction of the pump station and outflow improvements.

Documentation: The Grantee will submit: 1) the public notice of advertisement for the bid; 2) the bid package; and 3) a written notice of selected contractor(s).

Performance Standard: The Department's Grant Manager will review the documentation to verify that the deliverables have been completed as described above. Upon review and written acceptance by the Department's Grant Manager, the Grantee may proceed with payment request submittal.

Payment Request Schedule: The Grantee may submit a payment request for cost reimbursement following the conclusion of the task.

Task 3: Project Management

Deliverables: The Grantee will perform project management, to include field engineering services, construction observation, site meetings with construction contractor(s) and design professionals, and overall project coordination and supervision.

Documentation: The Grantee will submit interim progress status summaries including summary of inspection(s), meeting minutes and field notes, as applicable.

Performance Standard: The Department's Grant Manager will review the documentation to verify that the deliverables have been completed as described above. Upon review and written acceptance by the Department's Grant Manager, the Grantee may proceed with payment request submittal.

Payment Request Schedule: The Grantee may submit a payment request for cost reimbursement no more frequently than monthly.

Task 4: Construction

Deliverables: The Grantee will construct improvements to the pump house and related appurtenances in accordance with the construction contract documents.

Documentation: The Grantee will submit 1) a copy of the final design; 2) a signed acceptance of the completed work to date, as provided in the Grantee's Certification of Payment Request; and 3) a signed Engineer's Certification of Payment Request.

Performance Standard: The Department's Grant Manager will review the documentation to verify that the deliverables have been completed as described above. Upon review and written acceptance by the Department's Grant Manager, the Grantee may proceed with payment request submittal.

Payment Request Schedule: The Grantee may submit a payment request for cost reimbursement no more frequently than monthly.

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PROJECT TIMELINE & BUDGET DETAIL:

The tasks must be completed by, and all documentation received by, the corresponding task end date. Cost reimbursable grant funding must not exceed the budget amounts as indicated below.

Task No.	Task Title	Budget Category	Grant Amount	Task Start Date	Task End Date
1	Preconstruction Activities	Contractual Services	\$200,000	07/01/2022	10/31/2023
2	Bidding and Contractor Selection	Contractual Services	\$30,000	07/01/2022	10/31/2023
3	Project Management	Contractual Services	\$20,000	07/01/2022	10/31/2024
4	Construction	Contractual Services	\$370,000	07/01/2022	10/31/2024
		Miscellaneous/ Other Expenses	\$65,000		
Total:			\$750,000		

Note that, per Section 8.h. of Attachment 1 in the Agreement, authorization for continuation and completion of work and any associated payments may be rescinded, with proper notice, at the discretion of the Department if the Legislature reduces or eliminates appropriations. Extending the contract end date carries the risk that funds for this project may become unavailable in the future. This should be a consideration for the Grantee with this and future requests for extension.

Figure 1: Location map

